PROPOSED DINING HALL & SITE IMPROVEMENTS

285 PINE NOOK ROAD
DEERFIELD, MA 01342

APPLICANT:
EAGLEBROOK SCHOOL
271 PINE NOOK ROAD
DEERFIELD, MA 01342

SITE CIVIL ENGINEER:
PROTERRA DESIGN GROUP, LLC
4 BAY ROAD, BUILDING A, SUITE 200
HADLEY, MA 01035
CONTACT: JESSE MORENO, PE
PH: (413)320-4918

ARCHITECT:
WINDIGO ARCHITECTURE LLC
914 MT. KEMBLE AVENUE
MORRISTOWN, NJ 07960
CONTACT: ANGELA SUTTON, RA

SURVEYOR:
NORTHEAST SURVEY CONSULTANTS
3 FERRY STREET
EASTHAMPTON, MA 01027
CONTACT: BRIAN C. FRANETOVICH, PLS

LANDSCAPE ARCHITECT:
FUSS & O'NEILL
1550 MAIN STREET
SPRINGFIELD, MA 01103
CONTACT: ANDREW BOHNE, RLA

WETLAND SCIENTIST:
LUCAS ENVIRONMENTAL, LLC
500A WASHINGTON STREET
QUINCY, MA 02169
CONTACT: CHRIS M. LUCAS, PWS, CWS, RPSS

DRAWING LIST

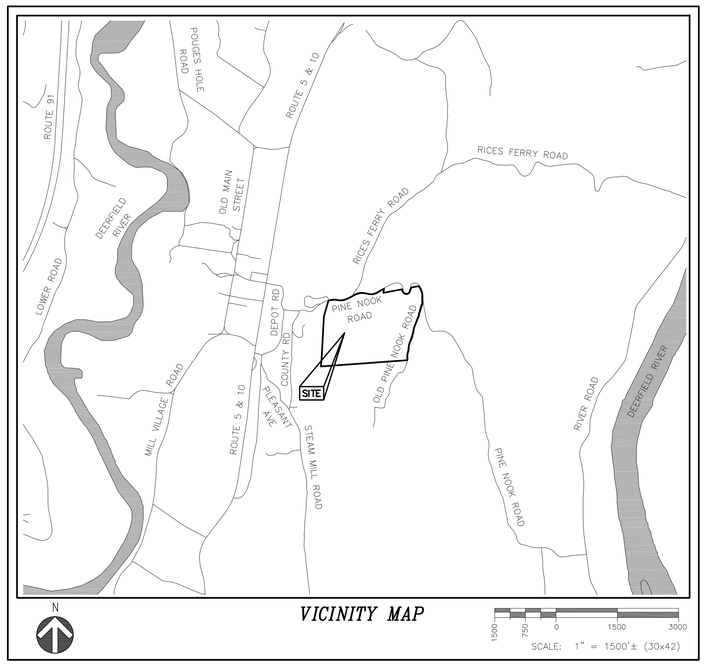
SHEET NUMBER	TITLE
C0.0	COVER SHEET
C0.1 – C0.7	EXISTING CONDITIONS
C1.1 – C1.2	GENERAL NOTES
C1.6 – C1.9	HAUL ROAD PLAN & PROFILE
C2.1	OVERALL SITE PLAN
C2.2 – C2.3	DETAILED SITE PLANS
C3.1 – C3.2	GRADING PLANS
C4.1 – C4.5	STORM SEWER PLANS
C5.1 – C5.2	DETAILED UTILITY PLANS
C5.3 – C5.5	SANITARY PLAN & PROFILE
C6.1 – C6.3	SWITCHGEAR PLANS
C6.4	SWITCHGEAR RETAINING WALL PLAN & PROFILE
C7.1 – C7.2	HAUL ROAD EROSION CONTROL PLANS
C7.3 – C7.4	EROSION CONTROL PLAN
C9.1 – C9.9	CIVIL SITE DETAILS

PLAN NOTES

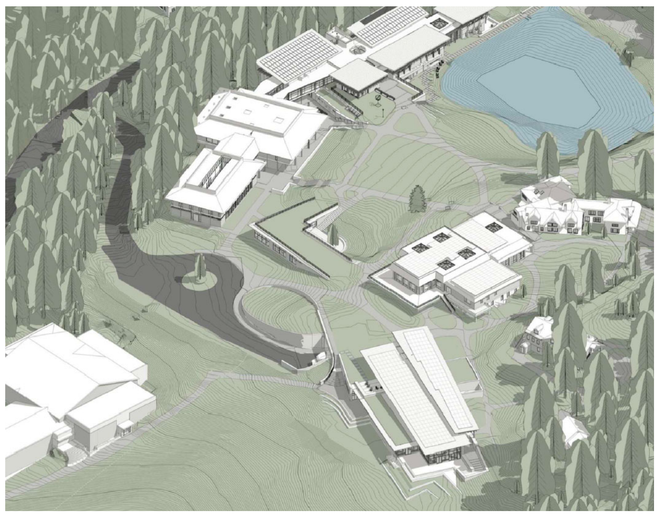
1. THE PLANS HEREIN ARE FOR NOTICE OF INTENT PERMITTING USE ONLY AND NOT FOR CONSTRUCTION.
2. ALL UNDERGROUND UTILITY INFORMATION WAS DETERMINED FROM SURFACE INVESTIGATIONS AND/OR EXISTING PLANS OF RECORD. THE CONTRACTOR SHALL LOCATE ALL UNDERGROUND UTILITIES IN THE FIELD PRIOR TO ANY SITE WORK. CALL THE FOLLOWING FOR ALL PRE-CONSTRUCTION NOTIFICATION 72-HOURS PRIOR TO ANY EXCAVATION ACTIVITY; DIG SAFE SYSTEM: 1-888-344-7233.



SITE RENDERING



VICINITY MAP



SITE RENDERING

NO.	DATE	SUBMISSION/REVISIONS	BY	CHK	APP'D
A	03/08/23	ISSUED FOR REVIEW	STAFF	JMM	JMM

STAMP:
COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023
Jesse Moreno, PE
Digitally signed by Jesse M. Moreno, PE
Date: 2023.03.09 15:16:30 -05'00'

PLAN TITLE:
PROPOSED DINING HALL & SITE IMPROVEMENTS
285 PINE NOOK ROAD
DEERFIELD, MA 01342

APPLICANT:
EAGLEBROOK SCHOOL
271 PINE NOOK ROAD
DEERFIELD, MA 01342

PREPARED BY:
ProTerra DESIGN GROUP, LLC
4 Bay Road, Bldg A Suite 200 Hadley, MA 01035
Ph:(413)320-4918

TITLE:
COVER SHEET

DRAWING NAME.: 22008_COVER.dwg	
JOB NUMBER: 22-008	DATE: MARCH 8, 2023
DESIGN: STAFF	CHECK: JMM
SCALE: AS NOTED	SHEET NO.: C0.0

NOTES:

1: FIELD SURVEY BY EDM TOTAL STATION IN AUGUST 2021, APRIL 2022, AUGUST 2022 AND SEPTEMBER 2022. JANUARY–MARCH 2023.

2: THE HORIZONTAL DATUM OF THIS PLAN IS NAD83, MASSACHUSETTS MAINLAND COORDINATE SYSTEM. THE VERTICAL DATUM IS NAVD88. BOTH WERE DERIVED FROM AN RTK GPS SURVEY PERFORMED ON SITE.

3: THE LOCATION OF UNDERGROUND UTILITY LINES SHOWN HEREON ARE FROM FIELD EVIDENCE, OPENING DRAIN & SEWER STRUCTURES AND INFORMATION PROVIDED BY GPRS, INC. THIS PLAN DOES NOT DEPICT THE EXACT LOCATION OF UNDERGROUND UTILITIES & MAY NOT SHOW ALL OF THE UTILITIES WHICH EXIST WITHIN THE PREMISES SURVEYED. PLEASE CONTACT DIG–SAFE AT 1–888–344–7233 BEFORE EXCAVATION.

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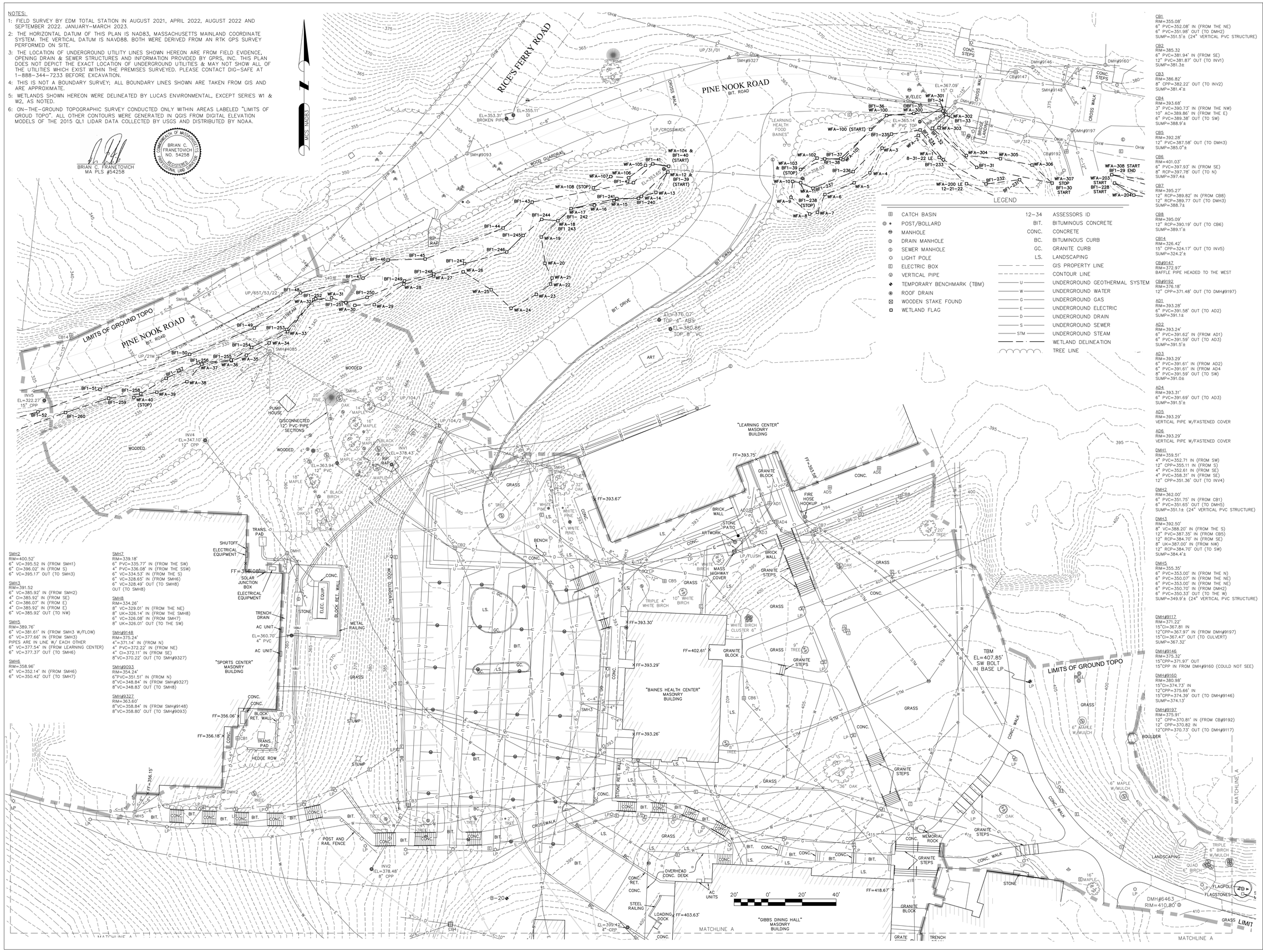
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6: ON–THE–GROUND TOPOGRAPHIC SURVEY CONDUCTED ONLY WITHIN AREAS LABELED "LIMITS OF GROUD TOPO". ALL OTHER CONTOURS WERE GENERATED IN QGIS FROM DIGITAL ELEVATION MODELS OF THE 2015 QL1 LIDAR DATA COLLECTED BY USGS AND DISTRIBUTED BY NOAA.

BRIAN C. FRANETOVICH
MA PLS #54258

COMMONWEALTH OF MASSACHUSETTS
BRIAN C. FRANETOVICH
NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

MCS NAD83



LEGEND

⊞	CATCH BASIN	12–34	ASSESSORS ID
◎ •	POST/BOLLARD	BIT.	BITUMINOUS CONCRETE
⊖	MANHOLE	CONC.	CONCRETE
◍	DRAIN MANHOLE	BC.	BITUMINOUS CURB
Ⓢ	SEWER MANHOLE	GC.	GRANITE CURB
☼	LIGHT POLE	LS.	LANDSCAPING
◫	ELECTRIC BOX		GIS PROPERTY LINE
Ⓥ	VERTICAL PIPE		CONTOUR LINE
◆	TEMPORARY BENCHMARK (TBM)	U	UNDERGROUND GEOTHERMAL SYSTEM
◉	ROOF DRAIN	W	UNDERGROUND WATER
⊠	WOODEN STAKE FOUND	G	UNDERGROUND GAS
◘	WETLAND FLAG	E	UNDERGROUND ELECTRIC
		D	UNDERGROUND DRAIN
		S	UNDERGROUND SEWER
		STM	UNDERGROUND STEAM
			WETLAND DELINEATION
			TREE LINE

CB1
RIM=355.08'
6" PVC=352.08' IN (FROM THE NE)
6" PVC=351.98' OUT (TO DMH2)
SUMP=351.5'± (24" VERTICAL PVC STRUCTURE)

CB2
RIM=385.32
6" PVC=381.94' IN (FROM SE)
12" PVC=381.87' OUT (TO INV1)
SUMP=381.3±

CB3
RIM=386.82'
8" CPP=382.22' OUT (TO INV2)
SUMP=381.4'±

CB4
RIM=393.68'
3" PVC=390.73' IN (FROM THE NW)
10" AC=389.86' IN (FROM THE E)
6" PVC=389.38' OUT (TO SW)
SUMP=388.9'±

CB5
RIM=392.28'
12" PVC=387.58' OUT (TO DMH3)
SUMP=385.0'±

CB6
RIM=401.03'
6" PVC=397.93' IN (FROM SE)
8" RCP=397.78' OUT (TO N)
SUMP=397.4±

CB7
RIM=395.27'
12" RCP=389.82' IN (FROM CB8)
12" RCP=389.77 OUT (TO DMH3)
SUMP=388.7±

CB8
RIM=395.09'
12" RCP=390.19' OUT (TO CB6)
SUMP=389.1'±

CB14
RIM=326.42'
15" CPP=324.17' OUT (TO INV5)
SUMP=324.2'±

CB@9147
RIM=372.97'
BAFFLE PIPE HEADED TO THE WEST

CB@9192
RIM=376.18'
12" CPP=371.48' OUT (TO DMH@9197)

AD1
RIM=393.28'
6" PVC=391.58" OUT (TO AD2)
SUMP=391.1±

AD2
RIM=393.24'
6" PVC=391.62' IN (FROM AD1)
6" PVC=391.59' OUT (TO AD3)
SUMP=391.5'±

AD3
RIM=393.29'
6" PVC=391.61' IN (FROM AD2)
6" PVC=391.61' IN (FROM AD4
8" PVC=391.59' OUT (TO SW)
SUMP=391.0±

AD4
RIM=393.31'
6" PVC=391.69' OUT (TO AD3)
SUMP=391.5'±

AD5
RIM=393.29'
VERTICAL PIPE W/FASTENED COVER

AD6
RIM=393.29'
VERTICAL PIPE W/FASTENED COVER

DMH1
RIM=359.51'
4" PVC=352.71 IN (FROM SW)
12" CPP=355.11 IN (FROM S)
4" PVC=352.61 IN (FROM SE)
4" PVC=358.31' IN (FROM SE)
12" CPP=351.36' OUT (TO INV4)

DMH2
RIM=362.00'
6" PVC=351.75' IN (FROM CB1)
6" PVC=351.65' OUT (TO DMH5)
SUMP=351.1± (24" VERTICAL PVC STRUCTURE)

DMH3
RIM=392.50'
8" VC=388.20' IN (FROM THE S)
12" PVC=387.35' IN (FROM CB5)
12" RCP=384.70' IN (FROM SE)
8" UK=387.00' IN (FROM NW)
12" RCP=384.70' OUT (TO SW)
SUMP=384.4'±

DMH5
RIM=355.35'
6" PVC=353.00' IN (FROM THE N)
6" PVC=350.07' IN (FROM THE NE)
6" PVC=353.00' IN (FROM THE NE)
6" PVC=350.70" IN (FROM DMH2)
6" PVC=350.33' OUT (TO THE W)
SUMP=349.9'± (24" VERTICAL PVC STRUCTURE)

DMH@9117
RIM=371.22'
15"CI=367.81 IN
12"CPP=367.97" IN (FROM DMH@9197)
15"CI=367.47' OUT (TO CULVERT)
SUMP=367.32'

DMH@9146
RIM=375.32'
15"CPP=371.97" OUT
15"CPP IN FROM DMH@9160 (COULD NOT SEE)

DMH@9160
RIM=380.98'
15"CI=374.73' IN
12"CPP=375.66" IN
15"CPP=374.39" OUT (TO DMH@9146)
SUMP=374.13'

DMH@9197
RIM=375.91'
12" CPP=370.81" IN (FROM CB@9192)
12" CPP=370.82 IN
12"CPP=370.73' OUT (TO DMH@9117)

SMH2
RIM=400.52'
6" VC=395.52 IN (FROM SMH1)
6" CI=396.02' IN (FROM S)
6" VC=395.17' OUT (TO SMH3)

SMH3
RIM=391.52
6" VC=385.92' IN (FROM SMH2)
4" CI=385.92' IN (FROM SE)
4" CI=386.07' IN (FROM E)
4" CI=385.92' IN (FROM E)
6" VC=385.92' OUT (TO NW)

SMH5
RIM=389.76'
6" VC=381.61' IN (FROM SMH3 W/FLOW)
6" VC=377.66" IN (FROM SMH3)
PIPES ARE IN LINE W/ EACH OTHER
6" VC=377.54' IN (FROM LEARNING CENTER)
6" VC=377.37' OUT (TO SMH6)

SMH6
RIM=358.96'
6" VC=352.14' IN (FROM SMH6)
6" VC=350.42' OUT (TO SMH7)

SMH7
RIM=339.18'
6" PVC=335.77' IN (FROM THE SW)
4" PVC=336.08' IN (FROM THE SSW)
6" VC=334.53" IN (FROM THE S)
6" VC=328.65" IN (FROM SMH8)
6" VC=328.49' OUT (TO SMH8)
OUT (TO SMH8)

SMH8
RIM=334.26'
8" VC=329.01' IN (FROM THE NE)
8" UK=326.14' IN (FROM THE SMH8)
6" VC=326.08" IN (FROM SMH7)
8" UK=326.01' OUT (TO THE SW)

SMH@9148
RIM=375.24'
4"=371.14' IN (FROM N)
4" PVC=372.22' IN (FROM NE)
4" CI=372.11' IN (FROM SE)
8"VC=370.22' OUT (TO SMH@9327)

SMH@9093
RIM=354.24'
6"PVC=351.51' IN (FROM N)
8"VC=348.84' IN (FROM SMH@9327)
8"VC=348.83' OUT (TO SMH8)

SMH@9327
RIM=363.60'
8"VC=358.84' IN (FROM SMH@9148)
8"VC=358.80' OUT (TO SMH@9093)

WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

CONSULTANT:
NORTHEAST SURVEY CONSULTANTS
3 Ferry Street
Studio 1 East
Easthampton, MA 01027
(413) 203–5144
northeastsurvey.com

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
0	3/7/23	FOR REVIEW

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
EXISTING CONDITIONS

DRAWING NUMBER
C0.1

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Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
0	3/7/23	FOR REVIEW

SMH1
RIM=413.15'
4" VC=409.50' IN (FROM THE S)
6" PVC=408.00' IN (FROM E)
6" PVC=408.85' IN (FROM NE)
3" UK=408.20' IN (FROM NE)
6" CI=408.00' OUT (TO SMH2)

SMH2
RIM=400.52'
6" VC=395.52 IN (FROM SMH1)
6" CI=396.02' IN (FROM S)
6" VC=395.17' OUT (TO SMH3)

SMH4
RIM=405.32'
4" CI=399.69' IN (FROM GIBBS BLDG)
6" CI=401.57' IN (FROM SMH1)
4" PVC=399.64' IN (FROM EAGLES NEST)
4" CI=399.45' IN (FROM THE S)
6" UK=397.51' OUT (TO SMH2)

DMH4
RIM=403.96'
10"CI=400.61' IN (FROM GIBBS BLDG)
10" UK=400.30' OUT (TO CB4)
SUMP=394.4'±

CB4
RIM=393.68'
3" PVC=390.73' IN (FROM THE NW)
10" AC=389.88' IN (FROM DMH4)
6" PVC=389.38' OUT (TO SW)
SUMP=388.9'±

CB9
RIM=419.59'
12" CPP=416.44' OUT (TO NE)
SUMP=415.8'± (24" VERTICAL PVC STRUCTURE)

CB10
RIM=419.35'
6" PVC=415.45' OUT (TO INV.3)
SUMP=415.4'±

CB11
RIM=329.15'
4" PLASTIC=328.75' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
6" PVC=328.45' OUT (TO CB12)

CB12
RIM=329.53'
6" PVC=327.68' IN (FROM CB11)
4" PLASTIC=327.80' IN (FROM CB15)
6" PVC=327.33' OUT (TO SE)
SUMP=326.5'±

AD7
RIM=419.46'
NO OUTLET
SUMP=418.5±

AD8
RIM=413.08'
3" PLASTIC=412.33' OUT (TO N)
SUMP=412.1'±

AD9
RIM=412.9'
2" VERTICAL PIPE

AD10
RIM=422.02'
3" PLASTIC=421.27' OUT (TO E)
SUMP=421.0'±

AD11
RIM=421.93'
3" PLASTIC=421.18' OUT (TO S)
SUMP=420.9'±

LEGEND

⊞	CATCH BASIN	12–34	ASSESSORS ID
◎ ●	POST/BOLLARD	BIT.	BITUMINOUS CONCRETE
Ⓜ	MANHOLE	CONC.	CONCRETE
◍	DRAIN MANHOLE	BC.	BITUMINOUS CURB
Ⓢ	SEWER MANHOLE	GC.	GRANITE CURB
☼	LIGHT POLE	LS.	LANDSCAPING
⊡	ELECTRIC BOX	——– – ——–	GIS PROPERTY LINE
⊗	VERTICAL PIPE	————	CONTOUR LINE
◆	TEMPORARY BENCHMARK (TBM)	——U——	UNDERGROUND GEOTHERMAL SYSTEM
◉	ROOF DRAIN	——W——	UNDERGROUND WATER
⊠	WOODEN STAKE FOUND	——G——	UNDERGROUND GAS
◘	WETLAND FLAG	——E——	UNDERGROUND ELECTRIC
		——D——	UNDERGROUND DRAIN
		——S——	UNDERGROUND SEWER
		——STM——	UNDERGROUND STEAM
		—— – · ——	WETLAND DELINEATION
		⌒⌒⌒⌒	TREE LINE

NOTES:
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BRIAN C. FRANETOVICH
MA PLS #54258

COMMONWEALTH OF MASSACHUSETTS
BRIAN C. FRANETOVICH
NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

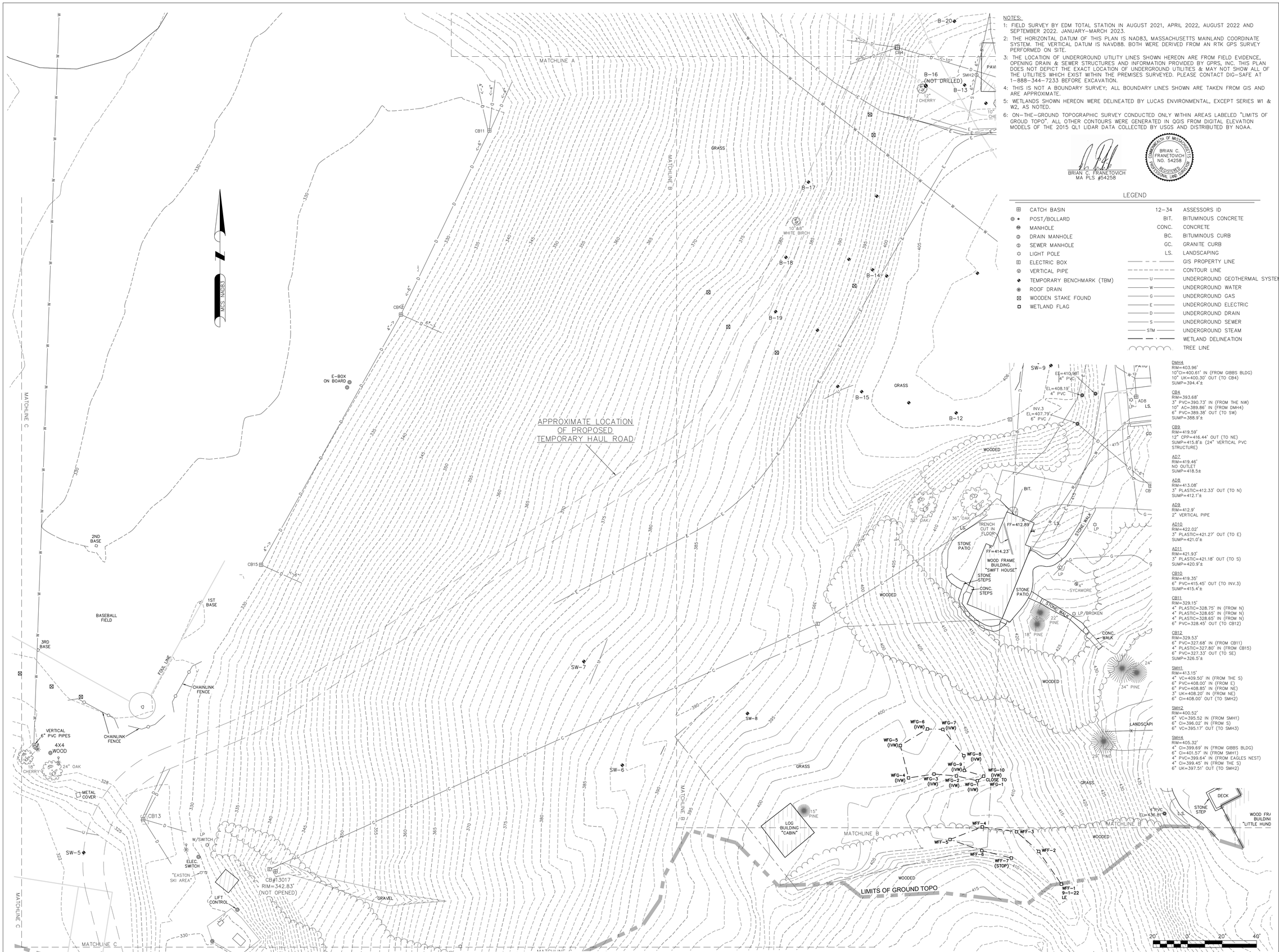
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
EXISTING CONDITIONS

DRAWING NUMBER
C0.2

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NOTES:

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MA PLS #54258

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NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

LEGEND

⊞	CATCH BASIN	12–34	ASSESSORS ID
◎ •	POST/BOLLARD	BIT.	BITUMINOUS CONCRETE
Ⓜ	MANHOLE	CONC.	CONCRETE
Ⓓ	DRAIN MANHOLE	BC.	BITUMINOUS CURB
Ⓢ	SEWER MANHOLE	GC.	GRANITE CURB
☼	LIGHT POLE	LS.	LANDSCAPING
⊟	ELECTRIC BOX	— — —	GIS PROPERTY LINE
⊘	VERTICAL PIPE	— — —	CONTOUR LINE
◆	TEMPORARY BENCHMARK (TBM)	—U—	UNDERGROUND GEOTHERMAL SYSTEM
◉	ROOF DRAIN	—W—	UNDERGROUND WATER
⊠	WOODEN STAKE FOUND	—G—	UNDERGROUND GAS
◘	WETLAND FLAG	—E—	UNDERGROUND ELECTRIC
		—D—	UNDERGROUND DRAIN
		—S—	UNDERGROUND SEWER
		—STM—	UNDERGROUND STEAM
		— · —	WETLAND DELINEATION
			TREE LINE

DMH4
RIM=403.96'
10"CI=400.61' IN (FROM GIBBS BLDG)
10" UK=400.30' OUT (TO CB4)
SUMP=394.4'±

CB4
RIM=393.68'
3" PVC=390.73' IN (FROM THE NW)
10" AC=389.86' IN (FROM DMH4)
6" PVC=389.38' OUT (TO SW)
SUMP=388.9'±

CB9
RIM=419.59'
12" CPP=416.44' OUT (TO NE)
SUMP=415.8'± (24" VERTICAL PVC STRUCTURE)

AD7
RIM=419.46'
NO OUTLET
SUMP=418.5±

AD8
RIM=413.08'
3" PLASTIC=412.33' OUT (TO N)
SUMP=412.1'±

AD9
RIM=412.9'
2" VERTICAL PIPE

AD10
RIM=422.02'
3" PLASTIC=421.27' OUT (TO E)
SUMP=421.0'±

AD11
RIM=421.93'
3" PLASTIC=421.18' OUT (TO S)
SUMP=420.9'±

CB10
RIM=419.35'
6" PVC=415.45' OUT (TO INV.3)
SUMP=415.4'±

CB11
RIM=329.15'
4" PLASTIC=328.75' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
6" PVC=328.45' OUT (TO CB12)

CB12
RIM=329.53'
6" PVC=327.68' IN (FROM CB11)
4" PLASTIC=327.80' IN (FROM CB15)
6" PVC=327.33' OUT (TO SE)
SUMP=326.5'±

SMH1
RIM=413.15'
4" VC=409.50' IN (FROM THE S)
6" PVC=408.00' IN (FROM E)
6" PVC=408.85' IN (FROM NE)
3" UK=408.20' IN (FROM NE)
6" CI=408.00' OUT (TO SMH2)

SMH2
RIM=400.52'
6" VC=395.52 IN (FROM SMH1)
6" CI=396.02' IN (FROM S)
6" VC=395.17' OUT (TO SMH3)

SMH4
RIM=405.32'
4" CI=399.69' IN (FROM GIBBS BLDG)
6" CI=401.57' IN (FROM SMH1)
4" PVC=399.64' IN (FROM EAGLES NEST)
4" CI=399.45' IN (FROM THE S)
6" UK=397.51' OUT (TO SMH2)

WINDIGO
ARCHITECTURE, LLC

914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

WWW.WINDIGODESIGN.COM

SITE CONSULTANT

CONSULTANT:
NORTHEAST SURVEY CONSULTANTS
3 Ferry Street
Studio 1 East
Easthampton, MA 01027
(413) 203-5144
northeastsurvey.com

Proposed Dining Hall
FOR
Eaglebrook School

LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

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SCALE
SEE PLAN

DRAWING NAME
EXISTING CONDITIONS

DRAWING NUMBER
C0.3

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SMH1
RIM=413.15'
4" VC=409.50' IN (FROM THE S)
6" PVC=408.00' IN (FROM E)
6" PVC=408.85' IN (FROM NE)
3" UK=408.20' IN (FROM NE)
6" CI=408.00' OUT (TO SMH2)

SMH2
RIM=400.52'
6" VC=395.52 IN (FROM SMH1)
6" CI=396.02' IN (FROM S)
6" VC=395.17' OUT (TO SMH3)

SMH4
RIM=405.32'
4" CI=399.89' IN (FROM GIBBS BLDG)
6" CI=401.57' IN (FROM SMH1)
4" PVC=399.64' IN (FROM EAGLES NEST)
4" CI=399.45' IN (FROM THE S)
6" UK=397.51' OUT (TO SMH2)

AD7
RIM=419.46'
NO OUTLET
SUMP=418.5±

AD8
RIM=413.08'
3" PLASTIC=412.33' OUT (TO N)
SUMP=412.1'±

AD9
RIM=412.9'
2" VERTICAL PIPE

AD10
RIM=422.02'
3" PLASTIC=421.27' OUT (TO E)
SUMP=421.0'±

AD11
RIM=421.93'
3" PLASTIC=421.18' OUT (TO S)
SUMP=420.9'±

DMH4
RIM=403.96'
10"CI=400.61' IN (FROM GIBBS BLDG)
10" UK=400.30' OUT (TO CB4)
SUMP=394.4'±

CB4
RIM=393.68'
3" PVC=390.73' IN (FROM THE NW)
10" AC=389.86' IN (FROM DMH4)
6" PVC=389.38' OUT (TO SW)
SUMP=388.9'±

CB9
RIM=419.59'
12" CPP=416.44' OUT (TO NE)
SUMP=415.8'± (24" VERTICAL PVC STRUCTURE)

CB10
RIM=419.35'
6" PVC=415.45' OUT (TO INV.3)
SUMP=415.4'±

CB11
RIM=329.15'
4" PLASTIC=328.75' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
6" PVC=328.45' OUT (TO CB12)

CB12
RIM=329.53'
6" PVC=327.68' IN (FROM CB11)
4" PLASTIC=327.80' IN (FROM CB15)
6" PVC=327.33' OUT (TO SE)
SUMP=326.5'±

CB13
RIM=326.82'
4" PVC=323.42' IN (FROM THE SE)
12" RCP=320.87" IN (FROM THE ESE)
6" PVC=321.82' IN (FROM THE NE)
6" PVC=321.82' IN (FROM THE NE)
12" RCP=320.62' OUT (TO INV6)

CB15
RIM=329.46'
4" PLASTIC=327.86' IN (FROM CB12)
6" PERF PVC=326.96' IN (FROM THE SE)
SUMP=325.6'±

NOTES:
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BRIAN C. FRANETOVICH
MA PLS #54258

COMMONWEALTH OF MASSACHUSETTS BRIAN C. FRANETOVICH NO. 54258 REGISTERED PROFESSIONAL LAND SURVEYOR

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Morristown, NJ 07960
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f 973.425.3921
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ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
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LEGEND

⊞	CATCH BASIN	12-34	ASSESSORS ID
◎ •	POST/BOLLARD	BIT.	BITUMINOUS CONCRETE
⊜	MANHOLE	CONC.	CONCRETE
⊕	DRAIN MANHOLE	BC.	BITUMINOUS CURB
Ⓢ	SEWER MANHOLE	GC.	GRANITE CURB
☆	LIGHT POLE	LS.	LANDSCAPING
⊡	ELECTRIC BOX		GIS PROPERTY LINE
Ⓥ	VERTICAL PIPE		CONTOUR LINE
◆	TEMPORARY BENCHMARK (TBM)	U	UNDERGROUND GEOTHERMAL SYSTEM
◉	ROOF DRAIN	W	UNDERGROUND WATER
⊠	WOODEN STAKE FOUND	G	UNDERGROUND GAS
□	WETLAND FLAG	E	UNDERGROUND ELECTRIC
		D	UNDERGROUND DRAIN
		S	UNDERGROUND SEWER
		STM	UNDERGROUND STEAM
			WETLAND DELINEATION
			TREE LINE

WETLAND W2 DELINEATED BY SWCA JULY, 2021

WETLAND W1 DELINEATED BY SWCA JULY, 2021

SMH1
RIM=413.15'
4" VC=409.50' IN (FROM THE S)
6" PVC=408.00' IN (FROM E)
6" PVC=408.85' IN (FROM NE)
3" UK=408.20' IN (FROM NE)
6" CI=408.00' OUT (TO SMH2)

SMH2
RIM=400.52'
6" VC=395.52' IN (FROM SMH1)
6" CI=396.02' IN (FROM S)
6" VC=395.17' OUT (TO SMH3)

SMH4
RIM=405.32'
4" CI=399.89' IN (FROM GIBBS BLDG)
6" CI=401.57' IN (FROM SMH1)
4" PVC=399.64' IN (FROM EAGLES NEST)
4" CI=399.45' IN (FROM THE S)
6" UK=397.51' OUT (TO SMH2)

DMH4
RIM=403.96'
10"CI=400.61' IN (FROM GIBBS BLDG)
10" UK=400.30' OUT (TO CB4)
SUMP=394.4'±

CB4
RIM=393.68'
3" PVC=390.73' IN (FROM THE NW)
10" AC=389.86' IN (FROM DMH4)
6" PVC=389.38' OUT (TO SW)
SUMP=388.9'±

CB9
RIM=419.59'
12" CPP=416.44' OUT (TO NE)
SUMP=415.8'± (24" VERTICAL PVC STRUCTURE)

CB10
RIM=419.35'
6" PVC=415.45' OUT (TO INV.3)
SUMP=415.4'±

CB11
RIM=329.15'
4" PLASTIC=328.75' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
4" PLASTIC=328.65' IN (FROM N)
6" PVC=328.45' OUT (TO CB12)

CB12
RIM=329.53'
6" PVC=327.68' IN (FROM CB11)
4" PLASTIC=327.80' IN (FROM CB15)
6" PVC=327.33' OUT (TO SE)
SUMP=326.5'±

AD7
RIM=419.46'
NO OUTLET
SUMP=418.5±

AD8
RIM=413.08'
3" PLASTIC=412.33' OUT (TO N)
SUMP=412.1'±

AD9
RIM=412.9'
2" VERTICAL PIPE

AD10
RIM=422.02'
3" PLASTIC=421.27' OUT (TO E)
SUMP=421.0'±

AD11
RIM=421.93'
3" PLASTIC=421.18' OUT (TO S)
SUMP=420.9'±

CB3
RIM=266.19'
6"PVC=263.64' (TO CB2)
6"PVC=263.54 (TO CB2)
6"PVC OUT=263.64' (TO NW)
PVC OUTs=263.39' (TO CB1)
SUMP=263.19'
WATER=263.59'

CB4
RIM=268.05'
6"PVC IN=265.65' (FROM S)
6"PVC IN=264.25' (FROM S)
6"PVC IN=264.25' (FROM SW)
6"PVC IN=264.45' (FROM W)
12"PVC OUT=263.60' (TO OUTLET)

NOTES:
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BRIAN C. FRANETOVICH
MA PLS #54258

COMMONWEALTH OF MASSACHUSETTS
BRIAN C. FRANETOVICH
NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

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FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

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WWW.WINDIGODESIGN.COM

SITE CONSULTANT

CONSULTANT:
NORTHEAST SURVEY CONSULTANTS
3 Ferry Street
Studio 1 East
Easthampton, MA 01027
(413) 203-5144
northeastsurvey.com

"LITTLE ROAD"

69-50
11 COUNTY ROAD
N/F
ALLEN CHASE
FOUNDATION

69-51
N/F
ALLEN CHASE
FOUNDATION

69-52
N/F
ALLEN CHASE
FOUNDATION

69-53
27 COUNTY RD.
N/F
ALLEN CHASE
FOUNDATION

"ALFOND AREA"

WETLAND W2 DELINEATED BY SWCA JULY, 2021

WETLAND W1 DELINEATED BY SWCA JULY, 2021

CONTOURS SHOWN BEHIND TREELINE IN THIS AREA CREATED FROM LIDAR DATA

CONTOURS SHOWN NORTH OF WETLAND LINE IN THIS AREA CREATED FROM LIDAR DATA

LIMITS OF GROUND TOPO

GRASS PLAYING FIELD

GOAL POSTS

GRAVEL PATH

PEASTONE

DRAIN PIPES ASSUMED TO RUN UNDER PEASTONE

DRAIN PIPE ASSUMED TO RUN UNDER PEASTONE

TRAIL

MATCHLINE D

MATCHLINE E

SEE OTHER SHEETS FOR NOTES.

BRIAN C. FRANETOVICH
MA PLS #54258

COMMONWEALTH OF MASSACHUSETTS
BRIAN C. FRANETOVICH
NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

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COMMONWEALTH OF MASSACHUSETTS
BRIAN C. FRANETOVICH
NO. 54258
REGISTERED PROFESSIONAL LAND SURVEYOR

LEGEND

⊞	CATCH BASIN	12–34	ASSESSORS ID
◎ •	POST/BOLLARD	BIT.	BITUMINOUS CONCRETE
⊕	MANHOLE	CONC.	CONCRETE
◎	DRAIN MANHOLE	BC.	BITUMINOUS CURB
◎	SEWER MANHOLE	GC.	GRANITE CURB
☼	LIGHT POLE	LS.	LANDSCAPING
⊡	ELECTRIC BOX		GIS PROPERTY LINE
⊘	VERTICAL PIPE		CONTOUR LINE
⚑	TEMPORARY BENCHMARK (TBM)	U	UNDERGROUND GEOTHERMAL SYSTEM
◉	ROOF DRAIN	W	UNDERGROUND WATER
⊠	WOODEN STAKE FOUND	G	UNDERGROUND GAS
□	WETLAND FLAG	E	UNDERGROUND ELECTRIC
		D	UNDERGROUND DRAIN
		S	UNDERGROUND SEWER
		STM	UNDERGROUND STEAM
			WETLAND DELINEATION
			TREE LINE

N
NAD83 MASS MAINLAND

PEASTONE
DRAIN PIPE ASSUMED TO RUN UNDER PEASTONE
CB2
RIM=266.42'
6"PVC IN=263.27' (FROM W)
(2) 6"PVC IN=261.62' (FROM CB3)
12"PVC OUT=261.47' (TO OUTLET)
SUMP=260.92'
CEDAR CLUSTER
E-BOX
SPIGOT
12" PVC EL=258.63'
CB3
RIM=266.19'
6"PVC=263.64' (TO CB2)
6"PVC=263.54 (TO CB2)
6"PVC OUT=263.64' (TO NW)
(2) 6"PVC OUTs=263.39' (TO CB1)
SUMP=263.19'
WATER=263.59'
STONE
STONE
GRAVEL PATH
STONE RET. WALLS
EL=253.42'
WETLAND W1 DELINEATED BY SWCA, JULY, 2021
LIMITS OF GROUND TOPO
WOODED
MATCHLINE E
MATCHLINE F
12" PVC EL=263.19'
CB4
RIM=268.05'
6"PVC IN=265.65' (FROM S)
6"PVC IN=264.25' (FROM S)
6"PVC IN=264.25' (FROM SW)
6"PVC IN=264.45' (FROM W)
12"PVC OUT=263.60' (TO OUTLET)
TRAIL
BOT. OF SLOPE
24" WHITE PINE
26" WHITE PINE
24" WHITE PINE
BLACK BIRCH
18"&15" DOUBLE OAK
8" TREE
GARAGE
LEDGE
APPROXIMATE LOCATION OF PROPOSED TEMPORARY HAUL ROAD
30" CMP EL=288.69'
30" CMP EL=291.87'
ATHLETIC FIELD
CB#13005
RIM=267.69'
4X4 WOOD POSTS
VERT. PIPES
2'X2' METAL COVER
CB#13000
RIM=259.09'
SHED
FENCE OPENING
WOOD STOCKADE FENCE
BIT

20' 0' 20' 40'

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ABBREVIATIONS

GENERAL

AADT	ANNUAL AVERAGE DAILY TRAFFIC
ABAN	ABANDON
ADJ	ADJUST
APPROX.	APPROXIMATE
A.C.	ASPHALT CONCRETE
ACCM PIPE	ASPHALT COATED CORRUGATED METAL PIPE
BIT.	BITUMINOUS
BC	BOTTOM OF CURB
BD.	BOUND
BL	BASELINE
BLDG	BUILDING
BM	BENCHMARK
BO	BY OTHERS
BOS	BOTTOM OF SLOPE
BR.	BRIDGE
CB	CATCH BASIN
CBCI	CATCH BASIN WITH CURB INLET
CC	CEMENT CONCRETE
CCM	CEMENT CONCRETE MASONRY
CEM	CEMENT
CI	CURB INLET
CIP	CAST IRON PIPE
CLF	CHAIN LINK FENCE
CL	CENTERLINE
CMP	CORRUGATED METAL PIPE
CSP	CORRUGATED STEEL PIPE
CO.	COUNTY
CONC	CONCRETE
CONT	CONTINUOUS
CONST	CONSTRUCTION
CR GR	CROWN GRADE
DHV	DESIGN HOURLY VOLUME
DI	DROP INLET
DIA	DIAMETER
DIP	DUCTILE IRON PIPE
DW	STEADY DON'T WALK - PORTLAND ORANGE
DWY	DRIVEWAY
ELEV (or EL.)	ELEVATION
EMB	EMBANKMENT
EOP	EDGE OF PAVEMENT
(E)	EXISTING
EXC	EXCAVATION
F&C	FRAME AND COVER
F&G	FRAME AND GRATE
FDN.	FOUNDATION
FLDSTN	FIELDSTONE
GAR	GARAGE
GD	GROUND
GG	GAS GATE
GI	GUTTER INLET
GIP	GALVANIZED IRON PIPE
GRAN	GRANITE
GRAV	GRAVEL
GRD	GUARD
HDW	HEADWALL
HMA	HOT MIX ASPHALT
HOR	HORIZONTAL
HYD	HYDRANT
INV	INVERT
JCT	JUNCTION
L	LENGTH OF CURVE
LB	LEACH BASIN
LP	LIGHT POLE
LT	LEFT
MAX	MAXIMUM
MB	MAILBOX
MH	MANHOLE
MHB	MASSACHUSETTS HIGHWAY BOUND
MIN	MINIMUM
NIC	NOT IN CONTRACT
NO.	NUMBER
PC	POINT OF CURVATURE
PCC	POINT OF COMPOUND CURVATURE
P.G.L.	PROFILE GRADE LINE
PI	POINT OF INTERSECTION
POC	POINT ON CURVE
POT	POINT ON TANGENT
PRC	POINT OF REVERSE CURVATURE
PROJ	PROJECT
(P)	PROPOSED
PSB	PLANTABLE SOIL BORROW
PT	POINT OF TANGENCY
PVC	POINT OF VERTICAL CURVATURE
PVI	POINT OF VERTICAL INTERSECTION
PVT	POINT OF VERTICAL TANGENCY
PVMT	PAVEMENT
PWW	PAVED WATER WAY

ABBREVIATIONS (cont.)

GENERAL

R	RADIUS OF CURVATURE
R&D	REMOVE AND DISPOSE
RCP	REINFORCED CONCRETE PIPE
RD	ROAD
RDWY	ROADWAY
REM	REMOVE
RET	RETAIN
RET WALL	RETAINING WALL
ROW	RIGHT OF WAY
RR	RAILROAD
R&R	REMOVE AND RESET
R&S	REMOVE AND STACK
RT	RIGHT
SB	STONE BOUND
SHLD	SHOULDER
SMH	SEWER MANHOLE
ST	STREET
STA	STATION
SSD	STOPPING SIGHT DISTANCE
SHLO	STATE HIGHWAY LAYOUT LINE
SW	SIDEWALK
T	TANGENT DISTANCE OF CURVE/TRUCK %
TAN	TANGENT
TEMP	TEMPORARY
TC	TOP OF CURB
TOS	TOP OF SLOPE
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
UP	UTILITY POLE
VAR	VARIES
VERT	VERTICAL
VC	VERTICAL CURVE
WCR	WHEEL CHAIR RAMP
WG	WATER GATE
WIP	WROUGHT IRON PIPE
WM	WATER METER/WATER MAIN
X-SECT	CROSS SECTION

LEGEND

N/F	NOW OR FORMERLY	D	DRAIN
IPF/IRF	IRON PIPE / ROD FOUND	C	COMMUNICATION
CONC.	CONCRETE	E	ELECTRIC
BIT.	BITUMINOUS CONCRETE	G	GAS
SGC	SLOPED GRANITE CURB	S	SEWER
VGC	VERTICAL GRANITE CURB	W	WATER
CC	CONCRETE CURB		METER
TBM	TEMP. BENCHMARK (TBM)		BOX
	LIGHT POST (LP)		MANHOLE
	UTILITY POLE (UP)		CATCH BASIN
	SIGN		GAS/WATER GATE
	BOLLARD/POST		HYDRANT
	FLAG POLE (FP)	BC	BIT. CURB
	DECIDUOUS TREE	CC	CONC. CURB
	CONIFEROUS TREE	VGC/GC	GRANITE CURB
	SHRUB/BUSH	SGC	SLOPED GRANITE CURB
	DETECTABLE WARNING		
	ROOF DRAIN (RD)		

- LOCUS PROPERTY LINE ±
- ABUTTING PROPERTY LINE ±
- GUARD RAIL
- U — UNDERGROUND UTILITY LINE
- MAJOR CONTOUR LINE
- MINOR CONTOUR LINE
- WHITE PAINTED LINE
- OHW — OVERHEAD WIRES
- PROPOSED ACCESSIBLE ROUTE
- PROPOSED CONCRETE
- PROPOSED REINFORCED TURF (FD ACCESS)
- SAWCUT LINE
- CROSSWALK
- TREE LINE, SHRUBS
- CHAIN LINK OR SEDIMENTATION FENCE

GENERAL NOTES

1. TOPOGRAPHICAL AND DETAIL INFORMATION IS FROM A SURVEY PERFORMED BY NORTHEAST SURVEY CONSULTANTS OF EASTHAMPTON, MASSACHUSETTS IN AUGUST 2021, APRIL 2022, & SEPTEMBER 2022. JANUARY–MARCH 2023. THE HORIZONTAL DATUM IS NAD 1983, THE VERTICAL DATUM IS NAVD88 AND ARE BASED UPON A DUAL FREQUENCY GPS RECEIVER AND POST–PROCESSING WITH CORS STATION DATA AND LIDAR SUPPLEMENTATION.
2. EXISTING GRADING AND LAYOUT PROVIDED BY LANDSCAPE ARCHITECT, FUSS & O'NEILL, PLAN TITLED "EXISTING CONDITIONS" DATED MARCH 7, 2023.
3. THE BASE SURVEY PLAN IS NOT THE RESULT OF A BOUNDARY SURVEY. PROPERTY LINES SHOWN ARE TAKEN FROM GIS AND ARE APPROXIMATE.
4. WETLANDS FLAG LOCATIONS BASED ON DELINEATION BY LUCAS ENVIRONMENTAL, LLC CONDUCTED ON AUGUST 31, 2022, SEPTEMBER 1, 2022 DECEMBER 15, 2022, DECEMBER 21, 2022, DECEMBER 22, 2022 & DECEMBER 28, 2022.
5. ADDITIONAL WETLANDS DELINEATED BY SWCA, JULY, 2021.
6. LOCATIONS OF EXISTING UNDERGROUND UTILITIES ARE SHOWN IN AN APPROXIMATE WAY BASED ON SURFACE OBSERVATION AT THE TIME OF SURVEY. THIS PLAN DOES NOT NECESSARILY DEPICT THE EXACT LOCATION OF THESE UTILITIES AND MAY NOT SHOW ALL OF THE UTILITIES WHICH EXIST WITHIN THE PREMISES SURVEYED. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES IN THE LIMIT OF WORK BEFORE COMMENCING WORK, AND SHALL BE RESPONSIBLE FOR ANY AND ALL DAMAGES CAUSED BY THE CONTRACTOR'S FAILURE TO EXACTLY LOCATE AND PRESERVE ANY AND ALL UNDERGROUND UTILITIES.
7. WHERE AN EXISTING UTILITY IS FOUND IN CONFLICT WITH THE PROPOSED WORK, THE LOCATION, ELEVATION, AND SIZE OF THE UTILITY SHALL BE ACCURATELY DETERMINED WITHOUT DELAY BY THE CONTRACTOR, AND THE INFORMATION FURNISHED TO THE ENGINEER FOR RESOLUTION OF THE CONFLICT.
8. THE CONTRACTOR SHALL ALTER THE MASONRY OF THE TOP SECTION OF ALL DRAINAGE STRUCTURES AS NECESSARY FOR CHANGES IN LINE AND GRADE, AND RESET ALL WATER, SEWER, AND DRAINAGE FRAMES, GRATES, AND BOXES TO THE PROPOSED FINISHED SURFACE GRADE. REQUIRED NEW MASONRY SHALL BE CLAY BRICK CONFORMING TO MASSDOT M4.05.2 OR EQUAL.
9. THE CONTRACTOR SHALL MAKE ALL ARRANGEMENTS FOR THE ALTERATION AND ADJUSTMENT OF GAS, ELECTRIC, TELEPHONE, AND ANY OTHER PRIVATE UTILITIES BY THE UTILITY COMPANIES.
10. TREES AND SHRUBS WITHIN THE LIMITS OF GRADING SHALL BE PROTECTED IN ACCORDANCE WITH THE TREE WARDEN AND REMOVED ONLY UPON APPROVAL OF THE TOWN. TREE PROTECTION PRIOR TO CONSTRUCTION SHALL BE BASED ON ANSI STANDARDS A300 PART 5 AND SHALL BE INSPECTED BY THE TREE WARDEN OR ENGINEER OF RECORD.
11. AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED BY THE CONTRACTOR TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE.
12. THE TERM "PROPOSED" (PROP OR P) MEANS WORK TO BE CONSTRUCTED USING NEW MATERIALS OR, WHERE APPLICABLE, RE–USING EXISTING MATERIALS IN SUITABLE CONDITION OR REFURBISHED AS IDENTIFIED AS "REMOVE AND RESET" (R&R) OR "REMOVE AND STACK" (R&S). THE TERM "EXISTING" (E) MEANS CONDITIONS CURRENTLY EXIST AT THE SITE.
13. ALL EXISTING GRANITE CURB, CURB CORNERS, & EDGING REMOVED WITHIN THE LIMITS OF WORK AND IN SUITABLE CONDITION AS DETERMINED BY THE ENGINEER, SHALL BE RE–USED IN THE PROPOSED WORK.
14. ALL EXISTING STATE, COUNTY, CITY, AND TOWN LOCATION LINES AND PRIVATE PROPERTY LINES HAVE BEEN ESTABLISHED FROM AVAILABLE INFORMATION AND THEIR EXACT LOCATIONS ARE NOT GUARANTEED.
15. CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING DIG SAFE BEFORE PERFORMING ANY EXCAVATION TO DETERMINE THE LOCATIONS OF ANY UNDERGROUND UTILITIES.
16. IN AREAS OF FULL DEPTH PAVEMENT AND SIDEWALK RECONSTRUCTION WHERE PROPOSED MEETS EXISTING PAVEMENT, THE EXISTING PAVEMENT SHALL BE SAWCUT TO OBTAIN A CLEAN VERTICAL FACE. ALL JOINTS BETWEEN EXISTING PAVEMENT TO REMAIN AND NEW PAVEMENT SHALL BE SEALED WITH HOT RUBBERIZED ASPHALT SEALER.
17. ANY EXISTING PAVEMENTS AND SOILS TO BE REMOVED WITHIN THE LIMITS OF CONSTRUCTION SHALL BE TRANSPORTED TO A LEGAL DISPOSAL LOCATION. NO STOCKPILING WILL BE ALLOWED WITHIN THE PROJECT LIMITS.
18. THE LAYOUT OF ALL PEDESTRIAN RAMPS SHALL CONFORM TO ADA/AAB STANDARDS AND CURRENT MASSDOT STANDARDS, AND DIRECTIVES.
19. ALL SOIL EROSION & SEDIMENT CONTROL DEVICES SHALL BE INSTALLED PRIOR TO ANY CONSTRUCTION ACTIVITY, AND BE MAINTAINED/SUPPLEMENTED AT THE CONTRACTOR'S EXPENSE THROUGH THE END OF JOB OR WHEN >75% OF THE VEGETATION IS ESTABLISHED WHICHEVER IS GREATER.
20. SILT SACKS SHALL BE INSTALLED AT ALL EXISTING CATCH BASINS WITHIN THE PROJECT LIMITS AND NEARBY VICINITY THAT MAY ACCEPT DRAINAGE OR RUNOFF FROM THE SITE.
21. THE FRAMES AND COVERS/GRATES FROM EXISTING DRAINAGE STRUCTURES TO BE REMOVED OR RESET.
22. ELEVATIONS ARE SHOWN FOR DESIGN PURPOSE ONLY. THE CONTRACTOR SHALL VERIFY ANY FIELD ADJUSTMENT AS REQUIRED TO CONFORM TO ADA/AAB AND CURRENT MASSDOT STANDARDS.
23. TIMING OF EARTHMOVING ACTIVITIES TO CORRESPOND TO LOW GROUNDWATER SEASON. ANY DEWATERING OR PERMITTING ASSOCIATED IS THE RESPONSIBILITY OF THE CONTRACTOR'S MEANS AND METHODS TO PERFORM THE WORK.
24. CONTRACTOR SHALL COORDINATE WITH PUBLIC SAFETY TO HAUL/PROCURE FILL MATERIALS BETWEEN 9am–2pm DAILY WEEKDAYS & THE EAGLEBROOK SCHOOL OUTSIDE PEAK HOUR SCHOOL TRAFFIC.

LAYOUT NOTES

1. LOCATE, PROTECT, AND MAINTAIN BENCH MARKS, MONUMENTS, CONTROL POINTS, AND PROJECT ENGINEERING REFERENCE POINTS. RE–ESTABLISH DISTURBED OR DESTROYED ITEMS AT CONTRACTORS EXPENSE.
2. THE CONTRACTOR IS RESPONSIBLE FOR HIS/HER LAYOUT OR CONTRACTING SEPARATELY WITH SURVEYOR OF RECORD.
3. EXAMINE THE AREAS AND CONDITIONS UNDER WHICH SITE WORK IS PERFORMED. REPORT ANY DISCREPANCIES WITH THE PLAN TO THE ENGINEER. DO NOT PROCEED WITH THE WORK UNTIL UNSATISFACTORY CONDITIONS ARE CORRECTED.
4. REPORT DISCREPANCIES IN DRAWINGS OR SPECIFICATIONS TO THE ENGINEER FOR CLARIFICATIONS AND ADJUSTMENTS BEFORE COMMENCING WORK. ANY DEVIATIONS OR CHANGES FROM THESE DRAWINGS WITHOUT WRITTEN ACCEPTANCE OF THE ENGINEER SHALL ABSOLVE THE ENGINEER OF ANY AND ALL RESPONSIBILITY OF SAID DEVIATION AND CHANGE.
5. WRITTEN DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS.
6. DIMENSIONS INDICATED ON PLANS ARE FOR HORIZONTAL CONTROL AND ARE ACCURATE IF MEASURED ON A LEVEL LINE. MEASURE HORIZONTAL CONTROL DIMENSIONS ON A LEVEL LINE, NOT PARALLEL WITH GROUND SLOPE.
7. ALL DIMENSIONS TAKEN FROM VERTICAL SURFACES, I.E. CURBS & WALLS, ARE UNDERSTOOD TO BE MEASURED FROM THE FACE OF THE VERTICAL ELEMENT UNLESS OTHERWISE SPECIFIED.
8. ALL ANGLES ASSUMED TO BE 90° UNLESS OTHERWISE SPECIFIED.

SITE PREPARATION

1. TEMPORARY EROSION CONTROL MEASURES IN ACCORDANCE WITH THE EROSION SEDIMENTATION CONTROL PLAN & ORDER OF CONDITIONS ISSUED BY THE CONSERVATION COMMISSION.
2. AREAS DESIGNATED FOR BORROW AREAS, EMBANKMENT, AND STRUCTURAL WORKS SHALL BE CLEARED, GRUBBED AND STRIPPED OF TOPSOIL.
3. VEGETATION AND OTHER MATERIAL SHALL BE CLEARED FROM POND AREA.
4. TOPSOIL SHALL BE STOCKPILED FOR FUTURE USE AS SPECIFIED.

EARTH FILL

1. FILL MATERIAL SHALL BE TAKEN FROM APPROVED BORROW AREAS AND SHALL BE FREE OF ROOTS, STUMPS, WOOD, RUBBISH, STONES GREATER THAN 6", FROZEN MATERIAL, AND OTHER OBJECTIONABLE MATERIALS.
2. FILL MATERIAL FOR CENTER OF EMBANKMENT SHALL CONFORM TO UNIFIED SOIL CLASSIFICATION GC, SC, CH, OR CL AND MUST HAVE AT LEAST 30% PASSING THE #200 SIEVE. CONSIDERATION MAY BE GIVEN TO THE USE OF OTHER MATERIALS IN THE EMBANKMENT IF DESIGNED BY A GEOTECHNICAL ENGINEER.
3. MATERIALS USED IN THE OUTER SHELL OF THE EMBANKMENT MUST HAVE THE CAPABILITY TO SUPPORT VEGETATION OF THE QUALITY REQUIRED TO PREVENT EROSION OF THE EMBANKMENT.
4. AREAS ON WHICH FILL IS TO BE PLACED SHALL BE SCARIFIED PRIOR TO PLACEMENT OF FILL. FILL MATERIALS SHALL BE PLACED IN MAXIMUM 8 INCH THICK (BEFORE COMPACTION) LAYERS WHICH ARE TO BE CONTINUOUS OVER THE ENTIRE LENGTH OF THE FILL. THE MOST PERMEABLE BORROW MATERIAL SHALL BE PLACED IN THE DOWNSTREAM PORTIONS OF THE EMBANKMENT. THE PRINCIPAL SPILLWAY MUST BE INSTALLED CONCURRENTLY WITH FILL PLACEMENT AND NOT EXCAVATED INTO THE EMBANKMENT.
5. WHEN REQUIRED BY THE REVIEWING AGENCY THE MINIMUM REQUIRED DENSITY SHALL NOT BE LESS THAN 95% OF MAXIMUM DRY DENSITY WITH A MOISTURE CONTENT WITHIN 2% OF THE OPTIMUM. EACH LAYER OF FILL SHALL BE COMPACTED AS NECESSARY TO OBTAIN THAT DENSITY, AND IS TO BE CERTIFIED BY THE ENGINEER AT THE TIME OF CONSTRUCTION. ALL COMPACTION IS TO BE DETERMINED BY AASHTO METHOD T–99 (STANDARD PROCTOR).
6. THE CORE OF THE EMBANKMENT SHALL BE PARALLEL TO THE CENTERLINE OF THE EMBANKMENT. THE TOP WIDTH OF THE CORE SHALL BE A MINIMUM OF FOUR FEET. THE HEIGHT SHALL EXTEND UP TO AT LEAST THE 10 YEAR WATER ELEVATION OR AS SHOWN ON THE PLANS. THE SIDE SLOPES SHALL BE 1 TO 1 OR FLATTER. THE CORE SHALL BE COMPACTED WITH CONSTRUCTION EQUIPMENT, ROLLERS, OR HAND TAMPERS TO ASSURE MAXIMUM DENSITY AND MINIMUM PERMEABILITY. IN ADDITION, THE CORE SHALL BE PLACED CONCURRENTLY WITH THE OUTER SHELL OF THE EMBANKMENT.

STRUCTURE BACKFILL

1. BACKFILL ADJACENT TO PIPES OR STRUCTURES SHALL BE OF THE TYPE AND QUALITY CONFORMING TO THAT SPECIFIED FOR THE ADJOINING FILL MATERIAL. THE FILL SHALL BE PLACED IN HORIZONTAL LAYERS NOT TO EXCEED FOUR INCHES IN THICKNESS AND COMPACTED BY HAND TAMPERS OR OTHER MANUALLY DIRECTED COMPACTION EQUIPMENT. THE MATERIAL NEEDS TO FILL COMPLETELY ALL SPACES UNDER AND ADJACENT TO THE PIPE.
2. AT NO TIME DURING THE BACKFILLING OPERATION SHALL DRIVEN EQUIPMENT BE ALLOWED TO OPERATE CLOSER THAN FOUR FEET, MEASURED HORIZONTALLY, TO ANY PART OF A STRUCTURE. UNDER NO CIRCUMSTANCES SHALL EQUIPMENT BE DRIVEN OVER ANY PART OF A CONCRETE STRUCTURE OR PIPE, UNLESS THERE IS A COMPACTED FILL OF 24" OR GREATER OVER THE STRUCTURE OR PIPE.

CARE OF WATER DURING CONSTRUCTION

1. ALL WORK ON PERMANENT STRUCTURES SHALL BE CARRIED OUT IN AREAS FREE FROM WATER. TEMPORARY DIKES, LEVEES, COFFERDAMS, DRAINAGE CHANNELS, AND STREAM DIVERSIONS NECESSARY TO PROTECT THE AREAS TO BE OCCUPIED BY THE PERMANENT WORKS SHALL BE INSTALLED, AS WELL AS PUMPING AND OTHER EQUIPMENT REQUIRED FOR REMOVAL OF WATER FROM VARIOUS PARTS OF THE WORK.
2. AFTER HAVING SERVED THEIR PURPOSE, ALL TEMPORARY PROTECTIVE WORKS SHALL BE REMOVED OR LEVELED AND GRADED TO THE EXTENT REQUIRED TO PREVENT OBSTRUCTION OF THE FLOW OF WATER TO THE SPILLWAY OR OUTLET WORKS.
3. STREAM DIVERSIONS SHALL BE MAINTAINED UNTIL THE FULL FLOW CAN BE PASSED THROUGH THE PERMANENT WORKS.

STABILIZATION AND EROSION CONTROL

1. ALL EXPOSED SURFACES OF THE EMBANKMENT, SPILLWAY, SPOIL AND BORROW AREAS, AND BERMS SHALL BE STABILIZED BY SEEDING, LIMING, FERTILIZING AND MULCHING IN ACCORDANCE WITH LOCAL NATURAL RESOURCES CONSERVATION SERVICE STANDARDS AND SPECIFICATIONS. A 4–INCH LAYER OF TOPSOIL SHALL BE PLACED ON THESE AREAS TO SUPPORT STABILIZING VEGETATION.
2. FILTER FABRIC PLACED BENEATH THE RIP–RAP SHALL MEET STATE OR LOCAL DEPARTMENT OF TRANSPORTATION REQUIREMENTS FOR A CLASS "C" FILTER FABRIC.

PROTERRA DESIGN GROUP, LLC ASSUMES NO RESPONSIBILITY FOR EXISTING UTILITY LOCATIONS (HORIZONTAL OR VERTICAL). THE EXISTING UTILITIES SHOWN ON THIS DRAWING HAVE BEEN PLOTTED FROM THE BEST AVAILABLE INFORMATION. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO FIELD VERIFY THE LOCATION OF ALL UTILITIES PRIOR TO THE COMMENCEMENT OF ANY CONSTRUCTION.

WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT
ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
Ph: (413)320–4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/06/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47315
REGISTERED PROFESSIONAL ENGINEER
3-6-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
GENERAL NOTES

DRAWING NUMBER
C1.1

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GENERAL:

1. PROTECTIONS
   A. PROVIDE PROTECTION NECESSARY TO PREVENT DAMAGE TO EXISTING IMPROVEMENTS, TREES OR VEGETATION.
   B. PROTECT IMPROVEMENTS ON ADJOINING PROPERTIES AND ON OWNER'S PROPERTY.
   C. RESTORE DAMAGED IMPROVEMENTS TO ORIGINAL CONDITION AS ACCEPTABLE TO PARTIES HAVING JURISDICTION.
   D. CONDUCT OPERATIONS TO ENSURE MINIMUM INTERFERENCE WITH OPERATIONS, STREETS, WALKS, AND OTHER ADJACENT FACILITIES. DO NOT CLOSE OR OBSTRUCT STREETS, WALKS, OR OTHER OCCUPIED OR USED FACILITIES WITHOUT PERMISSION FROM AUTHORITIES HAVING JURISDICTION. STREETS AND ROADWAYS SHALL BE THOROUGHLY CLEANED AND/OR SWEPT ON A DAILY BASIS OR MORE FREQUENTLY AS REQUIRED BY THE GOVERNING AUTHORITY.
2. UNLESS SPECIFIED OTHERWISE ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE MASSACHUSETTS HIGHWAY DEPARTMENT SPECIFICATIONS FOR HIGHWAYS AND BRIDGES AND/OR THE APPROPRIATE LOCAL AUTHORITIES.
3. ALL SLOPES, UNLESS OTHERWISE SPECIFIED, SHALL BE LOAMED AND SEEDED FOR STABILIZATION AS SOON AS POSSIBLE TO PREVENT EROSION INTO WETLAND RESOURCE AREAS, ABUTTING PROPERTIES, OR PUBLIC WAYS. EROSION CONTROL BLANKETS ARE REQUIRED FOR ALL 2H:1V SLOPES. SLOPES MAY NOT EXCEED 2H:1V. SEE "SEEDING" NOTE 3.
4. ANY DEVIATIONS, I.E. "FIELD CHANGES" FROM THE DESIGN PLAN(S) MUST BE APPROVED BY THE DESIGN ENGINEER IN WRITING. CONTRACTOR SHOULD BE AWARE THAT LOCAL AND STATE AUTHORITIES HAVE JURISDICTION AND APPROVALS MUST BE OBTAINED FROM THE APPROPRIATE AUTHORITY PRIOR TO THE IMPLEMENTATION OF THE "FIELD CHANGE." PROTERRA DESIGN GROUP, LLC ASSUMES NO LIABILITY OR RESPONSIBILITY FOR WORK ASSOCIATED WITH FIELD CHANGES COMPLETED WITHOUT REGARD TO THE "FIELD CHANGE" PROCEDURE.
5. PLEASE REFER TO THE GENERAL NOTES ON SHEET C-1 OF THIS SET.
6. RELOCATION OF ANY UTILITIES SHALL BE PERFORMED IN ACCORDANCE WITH THE PROVISIONS OF THE APPROPRIATE UTILITY COMPANY AND/OR REGULATORY AGENCY.
7. * * * DIG SAFE NOTE * * *
   ALL CONTRACTORS SHALL NOTIFY UTILITY COMPANIES AND GOVERNMENT AGENCIES, IN WRITING, OF THE INTENT TO EXCAVATE, BLAST, DEMOLISH, BORE, OR PERFORM OTHER EARTH MOVING OPERATIONS NO LESS THAN 72 HOURS AND NO MORE THAN 30 DAYS PRIOR TO THE COMMENCEMENT OF SUCH WORK (EXCLUSIVE OF SATURDAYS, SUNDAYS, AND LEGAL HOLIDAYS) OR CALL "DIG SAFE" AT 1-888-DIG-SAFE. **DEERFIELD WATER DEPARTMENT ARE NOT PART OF "DIG SAFE" AND MUST BE NOTIFIED SEPARATELY AND INDEPENDENTLY.**
8. LOCATION OF UNDERGROUND UTILITIES ON THIS PLAN ARE APPROXIMATE ONLY AND ARE BASED ON ACTUAL FIELD LOCATIONS OF VISIBLE STRUCTURES AND PLAN COMPILATIONS. EXISTING UTILITY LOCATIONS AND ELEVATIONS SHOWN SHALL BE VERIFIED BY CONTRACTOR PRIOR TO CONSTRUCTION. TEST PITS SHALL BE PERFORMED BY CONTRACTOR PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL INFORM THE DESIGN ENGINEER OF ANY DISCREPANCIES PRIOR TO THE START OF ANY CONSTRUCTION.
9. USE OF EXPLOSIVES
   A. COMPLY WITH ALL LAWS, RULES AND REGULATIONS OF FEDERAL, STATE, AND LOCAL AUTHORITIES AND INSURERS WHICH GOVERN STORAGE, USE, MANUFACTURE, SALE, HANDLING, TRANSPORTATION, LICENSING, OR OTHER DISPOSITION OF EXPLOSIVES. UTILIZE SPECIAL PRECAUTIONS FOR PROPER USE OF EXPLOSIVES TO PREVENT HARM TO HUMAN LIFE AND DAMAGE TO SURFACE STRUCTURES, ALL UTILITY LINES OR OTHER SUBSURFACE STRUCTURES. DO NOT CONDUCT BLASTING OPERATIONS UNTIL PERSONS IN VICINITY HAVE HAD AMPLE NOTICE AND HAVE REACHED POSITIONS OF SAFETY.
   B. BLASTING OF ANY ROCK SHALL REQUIRE APPROPRIATE APPROVALS FROM THE LOCAL FIRE DEPARTMENT, PLANNING BOARD, BUILDING INSPECTOR AND OTHER PERTINENT AGENCIES, PRIOR TO COMMENCEMENT OF WORK. ALL BLASTED ROCK OR OTHER EXCESS MATERIALS MUST BE REMOVED FROM THE SITE AND DISPOSED OF AT THE EXPENSE OF THE CONTRACTOR.
10. ADDITIONAL BENCHMARKS TO BE SET BY CONTRACTOR PRIOR TO CONSTRUCTION TO ENSURE QUALITY WORKMANSHIP.
11. THE PLANS CONTAINED HEREIN SHALL SUPERSEDE ANY AND ALL PREVIOUS DESIGN PREPARED FOR THE SUBJECT SITE. THE CONTRACTOR SHALL REVIEW ALL PLANS AND SPECIFICATIONS AND NOTIFY ENGINEER IMMEDIATELY OF ANY CONFLICTS OR DISCREPANCIES IN THE WORK. FAILURE TO DO SO DURING CONTRACT DOCUMENT REVIEW PERIOD WILL NEGATE ANY CLAIM DURING CONSTRUCTION FOR EXTRA TIME TO COMPLETE THE WORK.
12. ANY STILLING AND/OR DETENTION BASINS SHOULD RECEIVE PERIODIC MAINTENANCE DURING CONSTRUCTION TO REMOVE DEPOSITED SILTS AND DEBRIS TO ENSURE PROPERTY DRAINAGE AND SETTLING OF PARTICULATE MATTER.
13. ALL MANHOLE COVERS FOR CROSS-COUNTRY LOCATIONS OR IN PUBLIC GATHERING SHALL BE FITTED WITH BOLT LOCKS OR EQUIVALENT.
14. UNLESS OTHERWISE LABELED, ALL REINFORCED CONCRETE PIPE, RCP, SHALL BE CLASS V; ALL DUCTILE IRON PIPE SHALL BE CEMENT LINED CLASS 52; ALL PVC SEWER SHALL BE SDR 35; ALL CORRUGATED PLASTIC PIPE SHALL BE DOUBLE WALLED N-12 AS MANUFACTURED BY ADVANCED DRAINAGE SYSTEMS OR APPROVED EQUAL

SEQUENCE OF CONSTRUCTION:

1. CONTRACTOR MUST PROCEED WITH CONSTRUCTION ACCORDING TO THE CONDITIONS ESTABLISHED BY THE LOCAL ZONING BOARD, LOCAL CONSERVATION COMMISSION, AND ANY APPLICABLE ENVIRONMENTAL PERMITS INCLUDING BUT NOT LIMITED TO NPDES, ACOE 404, 401 WATER QUALITY CERTIFICATE. ORDERS OF CONDITIONS, CONDITIONS OF SPECIAL PERMIT, AND GROUND WATER DISCHARGE PERMIT. COMPLIANCE WITH THESE PERMITS SHALL BE A REQUIREMENT OF THE CONSTRUCTION AND SHALL BE REFERENCED IN ANY AGREEMENTS BETWEEN THE OWNER AND CONTRACTOR. THE CONTRACTOR MUST OBTAIN A COPY OF THE PERMITS PRIOR TO BIDDING.
2. A PRE-CONSTRUCTION MEETING WITH THE CONTRACTOR, OWNER'S REPRESENTATIVE, AND THE ENGINEER IS REQUIRED. **A PRE-CONSTRUCTION MEETING IS REQUIRED WITH THE GENERAL CONTRACTOR, SITE WORK CONTRACTOR, AND TOWN OF DEERFIELD FOR ALL WORK ASSOCIATED WITH STREET UTILITIES.**
3. ALL EROSION AND SEDIMENTATION CONTROL MEASURES MUST BE IN PLACE PRIOR TO CONSTRUCTION. MAINTAIN AND CLEAN TEMPORARY EROSION CONTROL DEVICES DURING CONSTRUCTION AS NECESSARY. INSPECT CHECK DAMS AND SILT FENCING DAILY. DURING CONSTRUCTION, ADDITIONAL EROSION CONTROL MEASURES MAY BE REQUIRED. PROVIDE MEASURES AS NECESSARY TO PREVENT SOIL FROM ENTERING OUTSIDE LIMIT OF WORK.
4. REMOVE EXISTING BUILDINGS AND RELATED APPURTENANCES AS INDICATED ON THE PLANS. REMOVE DEBRIS FROM THE SITE TO PREVENT MATERIAL FROM ENTERING RESOURCE AREAS OR ABUTTING PROPERTIES.
5. CONTRACTOR TO ABANDON EXISTING WELLS AND SEPTICS ON THE SITE IN ACCORDANCE WITH LOCAL BOARD OF HEALTH PROCEDURES.
6. ESTABLISH AREAS DESIGNATED FOR REVEGETATION. AERATE AND DECOMPACT SOIL IN PREPARATION FOR PLANTING. PROTECT PLANTED AREAS DURING CONSTRUCTION.
7. CLEAR, GRUB AND GRADE THE SITE AS SHOWN ON THE PLANS. PROVIDE PERMANENT VEGETATIVE COVER AND STABILIZATION IN AREAS WHERE NO FURTHER CONSTRUCTION IS PROPOSED. PROVIDE TEMPORARY EROSION PROTECTION IN AREAS SCHEDULED FOR FUTURE EXCAVATION. STOCKPILE SOIL AWAY FROM ENVIRONMENTALLY SENSITIVE AREAS AND DRAINAGE WAYS. STABILIZE DORMANT STOCKPILES UNTIL ACTIVE USE IS REQUIRED.
8. BEGIN CONSTRUCTION OF THE PROPOSED UTILITIES AND RELOCATION OF EXISTING UTILITIES. REMOVE CONSTRUCTION MATERIALS UPON COMPLETION OF WORK EACH DAY. DO NOT STORE CHEMICALS OR FUELS IN RESOURCE AREA BUFFERS OR NEAR PRIVATE WELLS. HAZARDOUS MATERIALS MUST BE SECURED ACCORDING TO ALL FEDERAL, STATE, AND LOCAL REGULATIONS. CONTRACTOR ENCOURAGED TO OBTAIN PHASE 1 ENVIRONMENTAL ASSESSMENT FROM OWNER.
9. STABILIZE ALL SLOPES AND RESTORE VEGETATION DISTURBED DURING CONSTRUCTION. PROTECT GRADED AREAS WITH STRAW MUCH OR EROSION CONTROL BLANKETS AS REQUIRED. REMOVE SEDIMENTATION CONTROL WHEN A GOOD VEGETATIVE COVER IS ESTABLISHED.

SITE WORK:

1. CAUTION - NOTICE TO CONTRACTOR

THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES AND, WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANY AT LEAST 72 HOURS (EXCLUSIVE OF SATURDAYS, SUNDAYS, AND LEGAL HOLIDAYS) PRIOR TO ANY EXCAVATION BLASTING, DEMOLITION, BORING, OR OTHER EARTH MOVING OPERATIONS TO REQUEST EXACT FIELD LOCATIONS OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS AT NO ADDITIONAL EXPENSE TO OWNER.

2. FILL MATERIAL
   A. ENSURE THAT AREAS TO BE FILLED ARE FREE OF STANDING WATER, FROST, FROZEN MATERIAL, TRASH, AND DEBRIS PRIOR TO FILL PLACEMENT.
   B. PLACE APPROPRIATE FILL MATERIAL AS DESIGNATED BY THE GEOTECHNICAL ENGINEER IN HORIZONTAL LAYERS NOT EXCEEDING EIGHT INCHES (8") IN LOOSE DEPTH AND COMPACT EACH LAYER AT OPTIMUM MOISTURE CONTENT OF FILL MATERIAL TO DENSITY EQUAL TO ORIGINAL ADJACENT GROUND, UNLESS SUBSEQUENT EXCAVATION FOR NEW WORK IS REQUIRED.
3. FINISH GRADING
   A. GRADE ALL AREAS WHERE FINISH GRADE ELEVATIONS ARE INDICATED ON DRAWINGS, OTHER THAN PAVED AREAS AND BUILDINGS, INCLUDING EXCAVATED AREAS, FILLED AND TRANSITION AREAS, AND LANDSCAPED AREAS. GRADED AREAS SHALL BE UNIFORM AND SMOOTH, FREE FROM DEBRIS, OR IRREGULAR SURFACE CHANGES. FINISHED SUBGRADE SURFACE SHALL NOT BE MORE THAN 0.10 FEET ABOVE OR BELOW ESTABLISHED SUBGRADE ELEVATIONS, AND ALL GROUND SURFACES SHALL VARY UNIFORMLY BETWEEN INDICATED ELEVATIONS. FINISH DITCHES SHALL BE GRADED TO ALLOW FOR PROPER DRAINAGE WITHOUT PONDING AND IN A MANNER THAT WILL MINIMIZE EROSION POTENTIAL
   B. GRADE SURFACE TO MATCH ADJACENT GRADES AND TO PROVIDE FLOW TO SURFACE DRAINAGE STRUCTURES, OR GRADE AS DESIGNATED ON THE PLANS AFTER FILL PLACEMENT AND COMPACTION.
4. THE CONTRACTOR IS RESPONSIBLE FOR GENERAL CLEANUP OF THE PROJECT ON A DAILY BASIS AND AT THE COMPLETION OF THE PROJECT. OPEN TRENCHES, DITCHES, EXCAVATIONS, ETC. SHALL NOT BE PERMITTED TO BE LEFT OPEN OVERNIGHT. CONTRACTOR WILL BACKFILL OR UTILIZE SUITABLE STEEL PLATES FOR THE SECURING OF THE PROJECT SITE PRIOR TO CEASING WORK EACH DAY.
5. APPROPRIATE TRAFFIC CONTROL, I.E. SIGNAGE, BARRICADES, AND OTHER MEANS, WILL BE SUPPLIED BY THE CONTRACTOR IN ACCORDANCE WITH ALL FEDERAL, STATE AND LOCAL AGENCIES.
6. UNDER NO CIRCUMSTANCES MAY ANY UTILITY, STRUCTURE, AND/OR REPAIR BE BACKFILLED UNLESS INSPECTED AND APPROVED BY THE CITY OFFICIALS AND/OR REPRESENTATIVE. RECEIPT OF APPROVAL TO BACKFILL WILL NOT RELEASE THE CONTRACTOR FROM ANY RESPONSIBILITY OR LIABILITY FOR PERFORMANCE TESTS REQUIRED AS PART OF THIS PROJECT.
7. PROPER SHORING AND TRENCH BOXES SHALL BE UTILIZED AS REQUIRED BY LOCAL, STATE, AND FEDERAL REGULATORY AGENCIES TO PROVIDE A SAFE WORKING ENVIRONMENT. SHORING SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER LICENSED IN THE STATE OF MASSACHUSETTS WITH EXPERIENCE IN SHORING DESIGN.
8. ALL UTILITIES DISTURBED DURING CONSTRUCTION SHALL BE REPAIRED BY THE CONTRACTOR AT NO ADDITIONAL COST TO THE OWNER.
9. WATER
   A. ALL WATER PIPING, VALVES, HYDRANTS, AND FITTINGS ETC. TO CONFORM TO LOCAL GUIDELINES OR AS DIRECTED BY THE WATER DEPARTMENT. CONSTRUCTION OF WATER LINE TO CONFORM TO ALL LOCAL AND STATE AGENCIES HAVING JURISDICTION.
   B. ALL WATER PIPE SHALL BE THICKNESS CLASS 52 DUCTILE IRON CEMENT LINED. ALL PIPES AND FITTINGS SHALL HAVE A CEMENT LINING TWICE THE THICKNESS SPECIFIED IN AWWA C104 AND SHALL HAVE A BITUMINOUS SEAL COAT APPLIES INSIDE AND OUTSIDE CONFORMING TO AWWA C104, "TYTON" OR MECHANICAL JOINTS ARE PERMITTED UNLESS OTHERWISE DIRECTED.
   C. REFER TO PLUMBING PLANS FOR WATER SERVICE, FIRE PROTECTION, AND SANITARY SEWER CONNECTIONS WITHIN 10' OF THE BUILDING UNLESS OTHERWISE NOTED.

SEWER NOTES

1. THESE NOTES ARE INTENDED TO SUPPLEMENT THE LOCAL REQUIREMENTS FOR MATERIALS AND WORKMANSHIP.
2. WATER AND SEWER MAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE LATEST LOCAL AND STATE CODES INCLUDING THE RECOMMENDATIONS OF THE AMERICAN WATER WORKS ASSOCIATION AND THE NEW ENGLAND INTERSTATE WATER POLLUTION CONTROL COMMISSION TECHNICAL REPORT 16. CONSTRUCTION SHALL PROCEED IN A WORKMANLIKE MANNER WITH STATE-OF-THE-ART CONSTRUCTION TECHNIQUES.
3. THE CONTRACTOR SHALL INSULATE WATER AND SEWER MAINS AS INDICATED ON THE PLANS OR WHEN DESIGN OR CONSTRUCTION ENCUMBRANCES DICTATE ALIGNMENT TO OCCUR ABOVE THE FROST LINE. PROCUREMENT AND INSTALLATION OF PIPE INSULATION SHALL CONFORM TO THE REQUIREMENTS LISTED IN THE LATEST MASS. HIGHWAY STANDARD SPECIFICATIONS FOR SECTION 301.60P AND MATERIAL SPECIFICATION M9.11.1. THE PIPE INSULATION SHALL BE PRE-MOLDED TYPE CELLULAR GLASS INSULATION WITH ALUMINUM JACKET CONFORMING TO THE LATEST REQUIREMENTS OF ASTM-522 OR APPROVED EQUAL.
4. THE CONTRACTOR SHALL FOLLOW ALTERNATE CONSTRUCTION PROCEDURES WHEN DESIGN OR CONSTRUCTION ENCUMBRANCES PREVENT HORIZONTAL SEPARATION OF 10 FEET OR THE ALTERNATE OF 18 INCHES OF VERTICAL SEPARATION BETWEEN WATER AND SEWER MAINS. IN AREAS WHERE THE ABOVE OFFSETS CANNOT BE MAINTAINED, THE WATER MAIN SHALL BE CONSTRUCTED WITH MEGA-LUG MECHANICAL TYPE FITTINGS OR APPROVED EQUAL FOR A DISTANCE OF 10-FEET ON EITHER SIDE OF THE CROSSING OR LATERAL ENCROACHMENT AND SHALL STRADDLE A FULL LENGTH OF CLASS 52 CEMENTED LINED DUCTILE IRON WATER PIPE.
5. EACH SEGMENT OF THE SEWER MAIN INCLUDING MANHOLES SHALL BE LEAK TESTED AND OBSERVED BY A REPRESENTATIVE OF THE CITY AND/OR ENGINEER IN ACCORDANCE WITH THE FOLLOWING PROCEDURES:

LEAKAGE TESTS

EXFILTRATION TEST:

A. PREPARATION OF TEST. AFTER THE MANHOLE HAD BEEN ASSEMBLED IN PLACE, ALL LIFTING HOLES AND THOSE EXTERIOR JOINTS WITHIN SIX FEET OF THE GROUND SURFACE SHALL BE FILLED AND POINTED WITH AN APPROVED NON-SHRINKING MORTAR. THE TEST SHALL BE MADE PRIOR TO PLACING THE SHELF AND INVERT AND BEFORE FILLING AND POINTING THE HORIZONTAL JOINTS BELOW THE 6-FOOT DEPTH LINE. IF THE GROUNDWATER TABLE HAS BEEN ALLOWED TO RISE ABOVE THE BOTTOM OF THE MANHOLE, IT SHALL BE LOWERED FOR THE DURATION OF THE TEST. ALL PIPES AND OTHER OPENINGS INTO THE MANHOLE SHALL BE SUITABLE PLUGGED AND PLUGS BRACED TO PREVENT BLOW OUT.

B. TEST PROCEDURE. THE MANHOLE SHALL THEN BE FILLED WITH WATER TO THE TOP OF THE CONE SECTION. IF THE EXCAVATION HAS NOT BEEN BACKFILLED AND OBSERVATION INDICATED NO VISIBLE LEAKAGE, THAT IS, NO WATER VISIBLY MOVING DOWN THE SURFACE OF THE MANHOLE, THE MANHOLE MAY BE CONSIDERED TO BE SATISFACTORILY WATERTIGHT. IF THE TEST AS DESCRIBED ABOVE IS UNSATISFACTORY AS DETERMINED BY THE ENGINEER OR IF THE MANHOLE EXCAVATION HAS BEEN BACKFILLED THE TEST SHALL BE CONTINUED. A PERIOD OF TIME MAY BE PERMITTED, IF THE CONTRACTOR WISHES, TO ALLOW FOR ABSORPTION.

C. AT THE END OF THIS PERIOD, THE MANHOLE SHALL BE REFILLED TO THE TOP OF THE CONE, IF NECESSARY, AND THE MEASURING TIME OF AT LEAST EIGHT HOURS BEGUN. AT THE END OF THE TEST PERIOD, THE MANHOLE SHALL BE REFILLED TO THE TOP OF THE CONE, MEASURING THE VOLUME OF WATER ADDED. THIS AMOUNT SHALL BE EXTRAPOLATED TO A 24-HOUR RATE AND THE LEAKAGE DETERMINED ON THE BASIS OF DEPTH. THE LEAKAGE FOR EACH MANHOLE SHALL NOT EXCEED ONE GALLON PER VERTICAL FOOT FOR A 24-HOUR PERIOD. IF THE TEST FAILS THIS REQUIREMENTS, BUT THE LEAKAGE DOES NOT EXCEED THREE GALLONS PER VERTICAL FOOT PER DAY, REPAIRS BY APPROVED METHODS MAY BE MADE AS DIRECTED BY THE ENGINEER TO BRING THE LEAKAGE WITHIN THE ALLOWABLE RATE ON ONE GALLON PER VERTICAL FOOT PER DAY. LEAKAGE DUE TO A DEFECTIVE SECTION OR JOINT OF EXCEEDING THE THREE-GALLON PER VERTICAL FOOT PER DAY RATE, SHALL BE CAUSE FOR THE REJECTION OF THE MANHOLE. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO UNCOVER, DISASSEMBLE, RECONSTRUCT OR REPLACE THE MANHOLE AS DIRECTED BY THE ENGINEER. THE MANHOLE SHALL THEN BE RE-TESTED AND, IF SATISFACTORY, INTERIOR JOINTS SHALL BE FILLED AND POINTED.

D. BACKFILLING. THE TEST MAY BE CONDUCTED EITHER BEFORE OR AFTER BACKFILLING AROUND THE MANHOLE. HOWEVER, IF THE CONTRACTOR ELECTS TO BACKFILL PRIOR TO TESTING, IT SHALL BE AT HIS OWN RISK AND IT SHALL BE INCUMBENT UPON THE CONTRACTOR TO DETERMINE THE REASON FOR ANY FAILURE OF THE TEST. NO ADJUSTMENT IN THE LEAKAGE ALLOWANCE WILL BE MADE FOR UNKNOWN CAUSES SUCH AS LEAKING PLUGS, ABSORPTION, ETC., I.E., IT WILL BE ASSUMED THAT ALL LOSS OF WATER DURING THE TEST IS A RESULT OF LEAKS THROUGH THE JOINTS OF THROUGH THE CONCRETE. FURTHERMORE, THE CONTRACTOR SHALL TAKE ANY STEPS NECESSARY TO ASSURE THE ENGINEER THAT THE WATER TABLE IS BELOW THE BOTTOM OF THE MANHOLE THROUGHOUT THE TEST.

VACUUM TEST:

A. THE VACUUM TESTING SYSTEM SHALL BE SUPPLIED BY NPC SYSTEMS, INC. OR EQUIVALENT AS APPROVED BY THE ENGINEER. THE TESTING SHALL BE DONE IMMEDIATELY AFTER ASSEMBLY OF THE MANHOLE AND BEFORE BACKFILLING. A 60 LB-FT. TORQUE WRENCH SHALL BE USED TO TIGHTEN EXTERNAL CLAMPS THAT SECURE THE TEST COVER TO THE TOP OF THE MANHOLE. ALL LIFT HOLES SHALL BE PLUGGED WITH A NON-SHRINKING MORTAR. THE CONTRACTOR SHALL PLUG THE PIPE OPENINGS, TAKING CARE TO SECURELY BRACE THE PLUGS AND THE PIPE TO PREVENT THE PLUGS FROM BEING DRAWN INTO THE MANHOLE.

B. A VACUUM OF 10 INCHES OF MERCURY, HG (4.9 PSI), SHALL BE DRAWN AND THE VACUUM PUMP SHUT OFF. THE MANHOLE PASSES THE TEST IF THE VACUUM REMAINS GREATER THAN OR EQUAL TO 9 INCHES HG (4.4 PSI) FOR A PERIOD GREATER THAN ONE MINUTE FOR MANHOLES UP TO 10 FEET DEEP; ONE MINUTE FIFTEEN SECONDS FOR MANHOLES 10-15 FEET DEEP; AND ONE MINUTE THIRTY SECONDS FOR MANHOLES 15-25 FEET DEEP.

C. IF THE MANHOLE FAILS THE INITIAL TEST, THE CONTRACTOR SHALL LOCATE THE LEAKS AND MAKE PROPER REPAIRS. LEAKS MAY BE FILLED WITH A WET SLURRY OF ACCEPTED QUICK SETTING MATERIAL. IF THE MANHOLE FAILS THE VACUUM TEST AGAIN, ADDITIONAL REPAIRS MUST BE MADE, AND THE MANHOLE MUST BE TESTED BY EXFILTRATION AS OUTLINED IN PARAGRAPH 3.03 (A). FIELD QUALITY CONTROL

LEAKAGE TESTS:

A. THE PIPELINES SHALL BE MADE AS NEARLY WATERTIGHT AS PRACTICABLE, AND LEAKAGE TESTS AND MEASUREMENTS SHALL BE MADE AFTER THE PIPELINE HAS BEEN BACKFILLED.

B. WHERE THE GROUNDWATER LEVEL IS MORE THAN 1 FT ABOVE THE TOP OF THE PIPE AT ITS UPPER END, THE CONTRACTOR SHALL CONDUCT EITHER INFILTRATION TESTS OR LOW PRESSURE AIR TESTS.

C. WHERE THE GROUNDWATER LEVEL IS LESS THAN 1 FT. ABOVE THE TOP OF THE PIPE AT ITS UPPER END, THE CONTRACTOR SHALL CONDUCT EITHER EXFILTRATION TESTS OR LOW PRESSURE AIR TESTS.

D. AT THE TIME OF THE TEST, THE CONTRACTOR SHALL DETERMINE THE GROUNDWATER ELEVATION FROM OBSERVATION WELLS, EXCAVATIONS OR OTHER MEANS, ALL SUBJECT TO REVIEW BY THE ENGINEER.

E. FOR MAKING THE LOW PRESSURE AIR TESTS, THE CONTRACTOR SHALL USE EQUIPMENT SPECIFICALLY DESIGNED AND MANUFACTURED FOR THE PURPOSE OF TESTING SEWER PIPELINES USING LOW PRESSURE AIR. THE EQUIPMENT SHALL BE PROVIDED WITH AN AIR REGULATORY VALVE OR AIR SAFETY SO SET THAT THE INTERNAL AIR PRESSURE IN THE PIPELINE CANNOT EXCEED 8 PSIG.

F. THE LEAKAGE TEST USING LOW PRESSURE AIR SHALL BE MADE ON EACH MANHOLE-TO-MANHOLE SECTION OF PIPELINE AFTER PLACEMENT OF THE BACKFILL.

G. PNEUMATIC PLUGS SHALL HAVE A SEALING LENGTH EQUAL TO OR GREATER THAN THE DIAMETER OF THE PIPE TO BE TESTED. PNEUMATIC PLUGS SHALL RESIST INTERNAL TEST PRESSURES WITHOUT REQUIRING EXTERNAL BRACING OR BLOCKING.

H. ALL AIR USED SHALL PASS THROUGH A SINGLE CONTROL PANEL.

I. LOW PRESSURE AIR SHALL BE INTRODUCED INTO THE SEALED LINE UNTIL THE INTERNAL AIR PRESSURE REACHES 4 PSIG. GREATER THAN THE MAXIMUM PRESSURE EXERTED BY THE GROUNDWATER THAT MAY BE ABOVE THE INVERT OF THE PIPE AT THE TIME OF THE TEST. HOWEVER, THE INTERNAL AIR PRESSURE IN THE SEALED LINE SHALL NOT BE ALLOWED TO EXCEED 8 PSIG. WHEN THE MAXIMUM PRESSURE EXERTED BY THE GROUNDWATER IS GREATER THAN 4 PSIG., THE CONTRACTOR SHALL CONDUCT ONLY AN INFILTRATION TEST.

J. AT LEAST TWO MINUTES SHALL BE ALLOWED FOR THE AIR PRESSURE TO STABILIZE IN THE SECTION UNDER TEST. AFTER THE STABILIZATION PERIOD, THE LOW PRESSURE AIR SUPPLY HOSE SHALL BE QUICKLY DISCONNECTED FROM THE CONTROL PANEL. THE TIME REQUIRED IN MINUTES FOR THE PRESSURE IN THE SECTION UNDER TEST TO DECREASE FROM 3.5 TO 2.5 PSIG (GREATER THAN THE MAXIMUM PRESSURE EXERTED BY GROUNDWATER THAT MAY BE ABOVE THE INVERT OF THE PIPE) SHALL NOT BE LESS THAN THAT SHOWN IN THE FOLLOWING TABLE:

PIPE DIAMETER IN INCHES VS. MINUTES

6"	5.0 MIN. 40 SEC.
8"	7.0 MIN. 34 SEC.
10"	9.0 MIN. 26 SEC.
12"	11.0 MIN. 20 SEC.
15"	14.0 MIN. 10 SEC.
18"	17.0 MIN. 0 SEC.
21"	19.0 MIN. 50 SEC.
24"	22.0 MIN. 40 SEC.
27"	25.0 MIN. 30 SEC.

K. FOR MAKING THE INFILTRATION AND EXFILTRATION TESTS, THE CONTRACTOR SHALL FURNISH SUITABLE TEST PLUGS, WATER PUMPS, AND APPURTENANCES, AND ALL LABOR REQUIRED TO PROPERLY CONDUCT THE TESTS ON SECTIONS OF ACCEPTABLE LENGTH.

L. FOR MAKING THE INFILTRATION TESTS, UNDERDRAINS, IF USED, SHALL BE PLUGGED AND OTHER GROUNDWATER DRAINAGE SHALL BE STOPPED TO PERMIT THE GROUNDWATER TO RETURN TO ITS NORMAL LEVEL INSOFAR AS PRACTICABLE

M. UPON COMPLETION OF A SECTION OF THE SEWER, THE CONTRACTOR SHALL DEWATER IT AND CONDUCT A SATISFACTORY TEST TO MEASURE THE INFILTRATION FOR AT LEAST 24 HOURS. THE AMOUNT OF INFILTRATION, INCLUDING MANHOLES, TEES, AND CONNECTIONS, SHALL NOT EXCEED 200 GAL. PER INCH DIAMETER PER MILE OF SEWER PER 24 HOURS.

N. FOR MAKING THE EXFILTRATION TESTS, THE SEWERS SHALL BE SUBJECTED TO AN INTERNAL PRESSURE BY PLUGGING THE PIPE AT THE LOWER END AND THEN FILLING THE PIPELINES AND MANHOLES WITH CLEAN WATER TO A HEIGHT OF 2 FT. ABOVE THE TOP OF THE SEWER AT ITS UPPER END. WHERE CONDITIONS BETWEEN MANHOLES, MAY RESULT IN TEST PRESSURES WHICH WOULD CAUSE LEAKAGE AT THE STOPPERS IN BRANCHES, PROVISIONS SHALL BE MADE BY SUITABLE TIES, BRACES, AND WEDGES TO SECURE THE STOPPERS AGAINST LEAKAGE RESULTING FROM THE TEST PRESSURE.

O. THE RATE OF LEAKAGE FROM THE SEWERS SHALL BE DETERMINED BY MEASURING THE AMOUNT OF WATER REQUIRED TO MAINTAIN THE LEVEL 2 FT. ABOVE THE TOP OF THE PIPE.

P. LEAKAGE FROM THE SEWERS UNDER TEST SHALL NOT EXCEED THE REQUIREMENTS FOR LEAKAGE INTO SEWERS AS HEREIN BEFORE SPECIFIED.

Q. THE SEWERS SHALL BE TESTED BEFORE ANY CONNECTIONS ARE MADE TO BUILDINGS.

R. THE CONTRACTOR SHALL CONSTRUCT WEIRS OR OTHER MEANS OF MEASUREMENTS AS MAY BE REQUIRED.

S.

T. SUITABLE BULKHEADS SHALL BE INSTALLED, AS REQUIRED, TO PERMIT THE TEST OF THE SEWER.

U. SHOULD THE SECTIONS UNDER TEST FAIL TO MEET THE REQUIREMENTS, THE CONTRACTOR SHALL DO ALL WORK OF LOCATING AND REPAIRING LEAKS AND RETESTING AS THE ENGINEER MAY REQUIRE WITHOUT ADDITIONAL COMPENSATION.

V. IF, IN THE JUDGMENT OF THE ENGINEER, IT IS IMPRACTICABLE TO FOLLOW THE FOREGOING PROCEDURES FOR ANY REASON, ACCEPTABLE MODIFICATIONS IN THE PROCEDURES SHALL BE MADE AS REQUIRED, BUT IN ANY EVENT, THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ULTIMATE TIGHTNESS OF THE LINE WITHIN THE ABOVE TEST REQUIREMENTS.

CONCRETE AND REINFORCING STEEL NOTES:

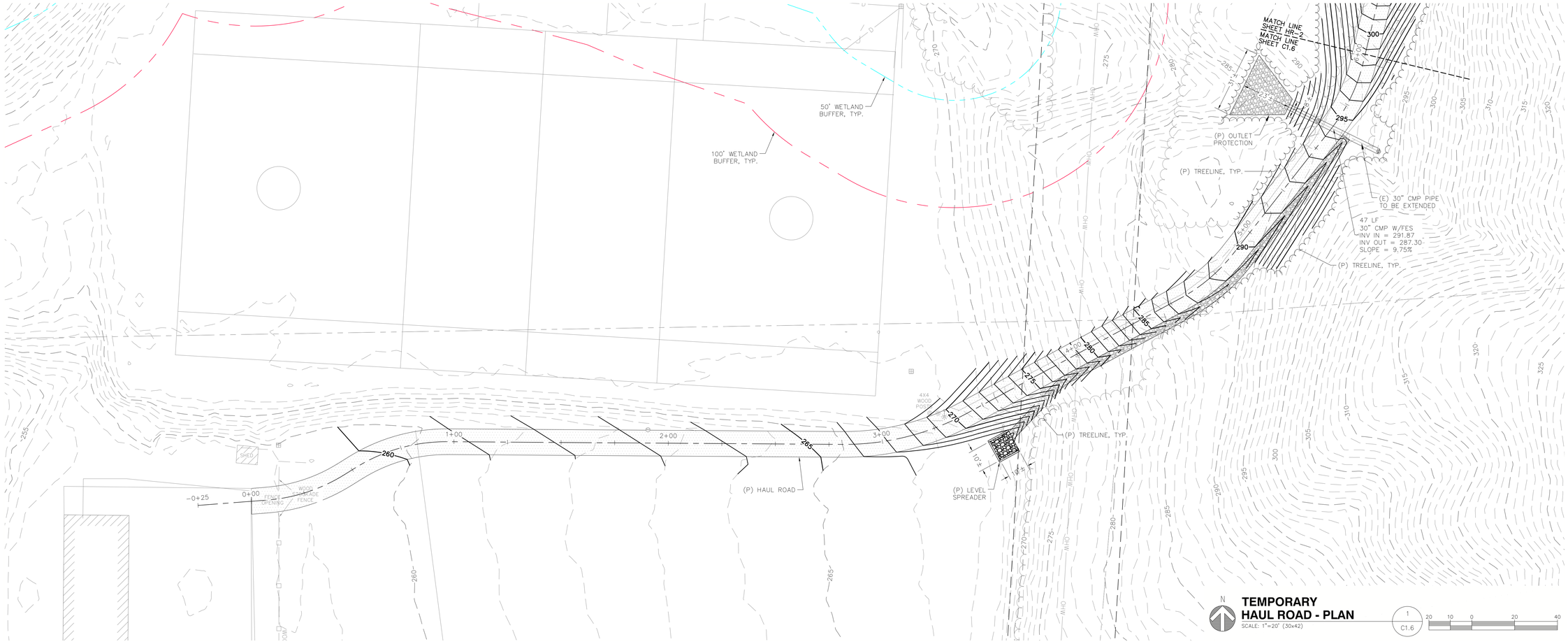
1. GENERAL
   - ALL STRUCTURAL DRAWINGS ARE TO BE USED WITH THE ENTIRE SET OF DRAWINGS.
   - ALL SAFETY REGULATIONS ARE TO BE STRICTLY FOLLOWED. METHODS OF CONSTRUCTION AND ERECTION OF STRUCTURAL MATERIALS IS THE CONTRACTOR'S RESPONSIBILITY.
   - THE CONTRACTOR IS RESPONSIBLE FOR DISSEMINATION OF ALL REVISIONS AND REQUIREMENTS TO THE SUBCONTRACTORS.
   - REASONABLE CARE HAS BEEN TAKEN IN THE PREPARATION OF ALL DRAWINGS AND SPECIFICATIONS. HOWEVER THE ENGINEER DOES NOT GUARANTEE AGAINST HUMAN ERROR AND FOR THAT REASON IT IS IMPERATIVE THAT THE CONTRACTOR SHALL CHECK ALL DIMENSIONS AND DETAILS AND MUST VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. ALL DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING.
   - PRESUMED ALLOWABLE SOIL BEARING CAPACITY IS 2000 PSF. VERIFY IN THE FIELD PRIOR TO THE START OF CONSTRUCTION.
2. CODE CONFORMANCE

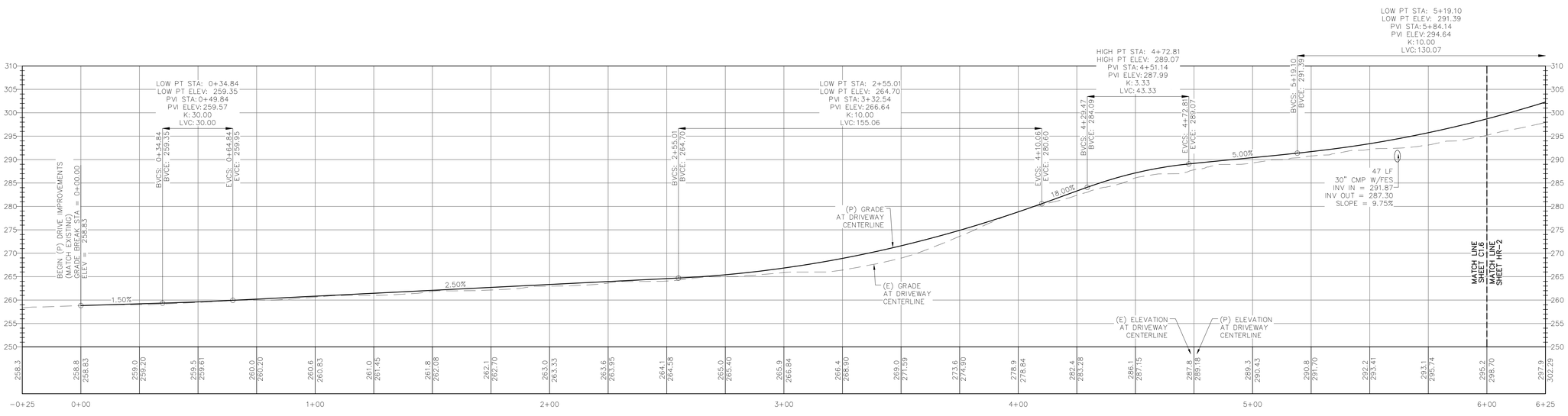
   TO COMPLY WITH THE LATEST RECOMMENDATIONS OF THESE STANDARDS.

   - ACI 301- "STRUCTURAL CONCRETE FOR BUILDINGS"
   - ACI 315- "DETAILING CONCRETE WORK"
   - ACI 318- "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
   - ACI 322- "BUILDING CODE REQUIREMENTS FOR STRUCTURAL PLAIN CONCRETE"
   - ACI 347- "FORM WORK"
3. MATERIALS

   CONCRETE:

   - APPROVED, READY MIXED CONCRETE HAVING AN MINIMUM COMPRESSIVE STRENGTH (f'c) OF 4,500 PSI AT 28 DAYS WITH 3/4"AGGREGATE MAX. (3/8" AGGREGATE MAY BE USED FOR SIDEWALK MIX), SLUMP 3-5 INCHES UNLESS OTHERWISE NOTED
   - PROVID





TEMPORARY HAUL ROAD - PROFILE
SCALE: 1"=20' HORIZONTAL, 1"=10' VERTICAL
2 C1.6
0 5 10
20 10 0

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WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
Ph: (413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/06/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

JESSE M. MORENO
CIVIL
No. 47815
REGISTERED PROFESSIONAL ENGINEER
COMMONWEALTH OF MASSACHUSETTS
3-6-2023

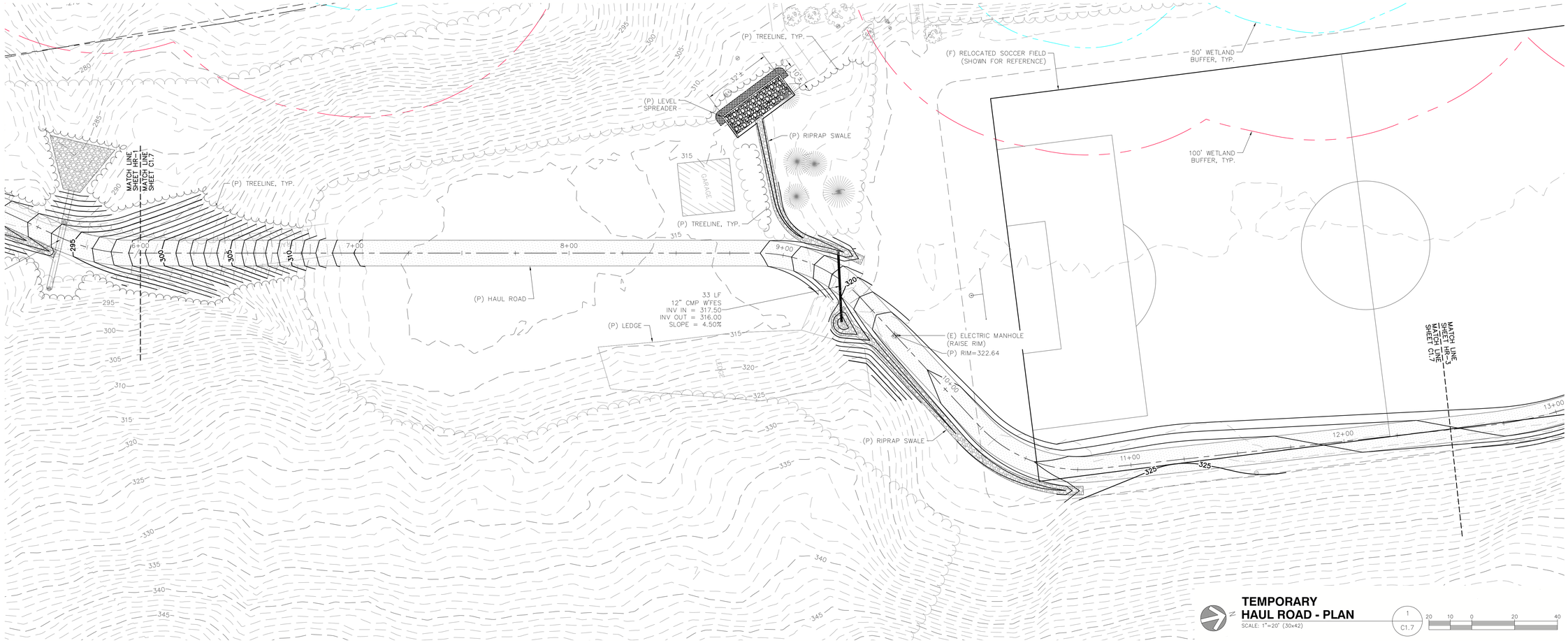
PROJECT NUMBER
22008

SCALE
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DRAWING NAME
TEMPORARY HAUL ROAD PLAN & PROFILE

DRAWING NUMBER
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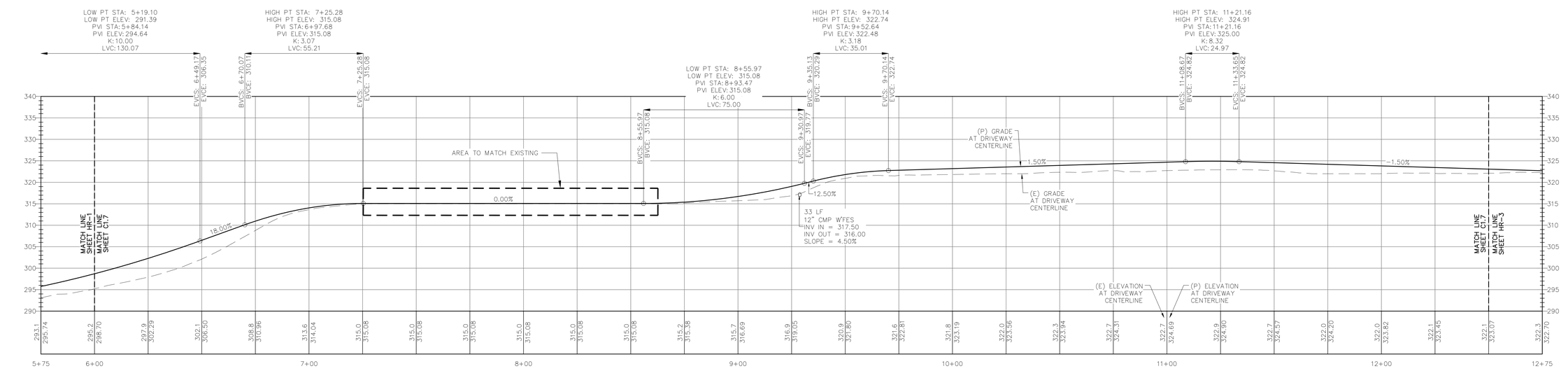


TEMPORARY HAUL ROAD - PLAN

SCALE: 1"=20' (30x42)

1 / C1.7

20 10 0 20 40



TEMPORARY HAUL ROAD - PROFILE

SCALE: 1"=20' HORIZONTAL
1"=10' VERTICAL

2 / C1.7

10 5 0

20 10 0

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ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

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DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
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Ph: (413)320-4918

Proposed Dining Hall
FOR
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LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

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	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

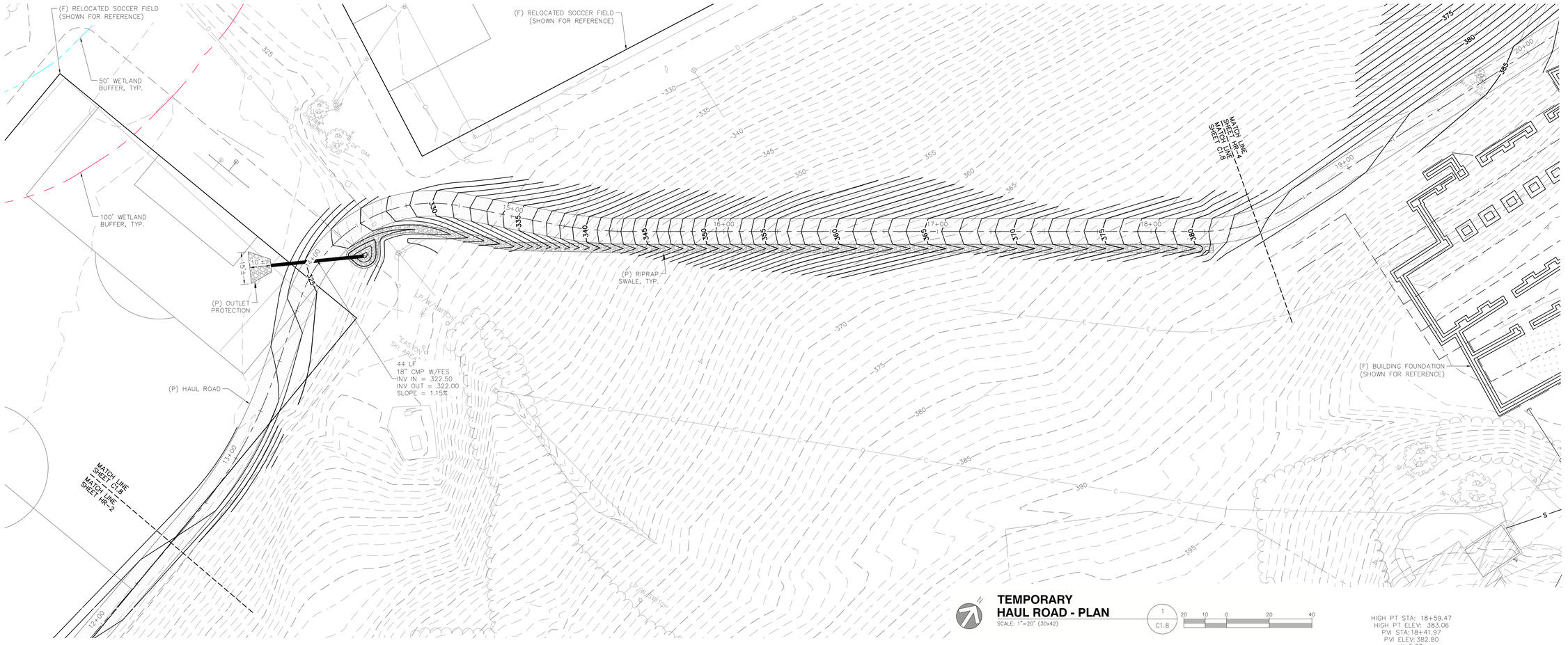
COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

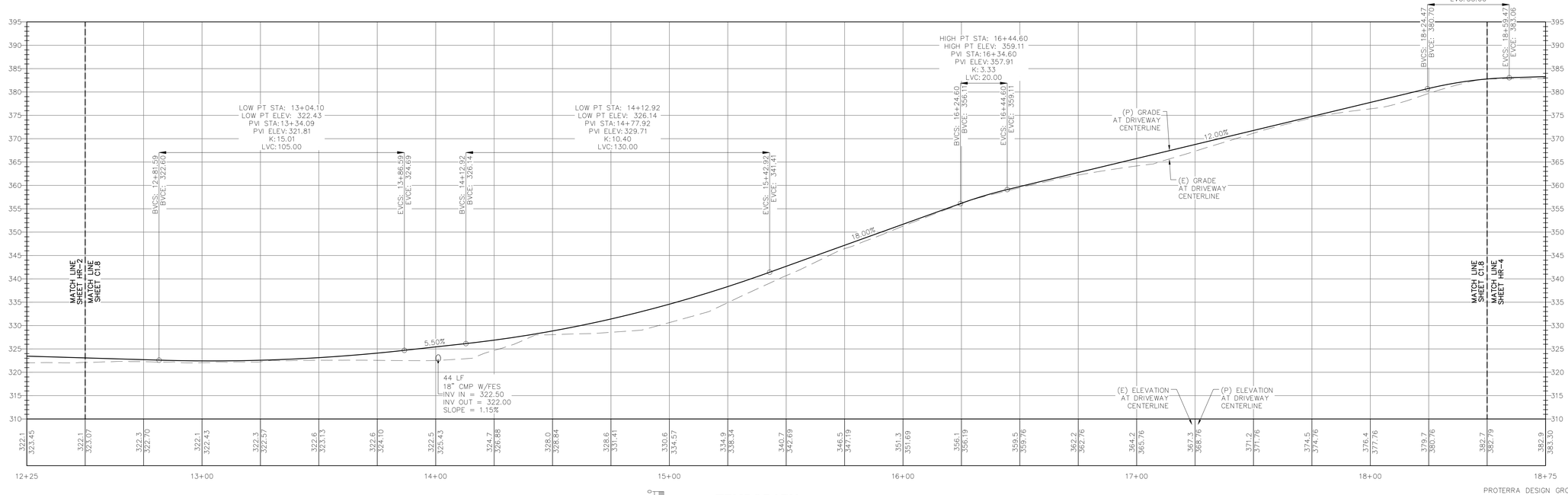
PROJECT NUMBER
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SCALE
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DRAWING NAME
TEMPORARY HAUL ROAD PLAN & PROFILE

DRAWING NUMBER
C1.7





TEMPORARY HAUL ROAD - PROFILE
SCALE: 1"=20' HORIZONTAL
1"=10' VERTICAL
2
C1.8
20 10 0
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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47219
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

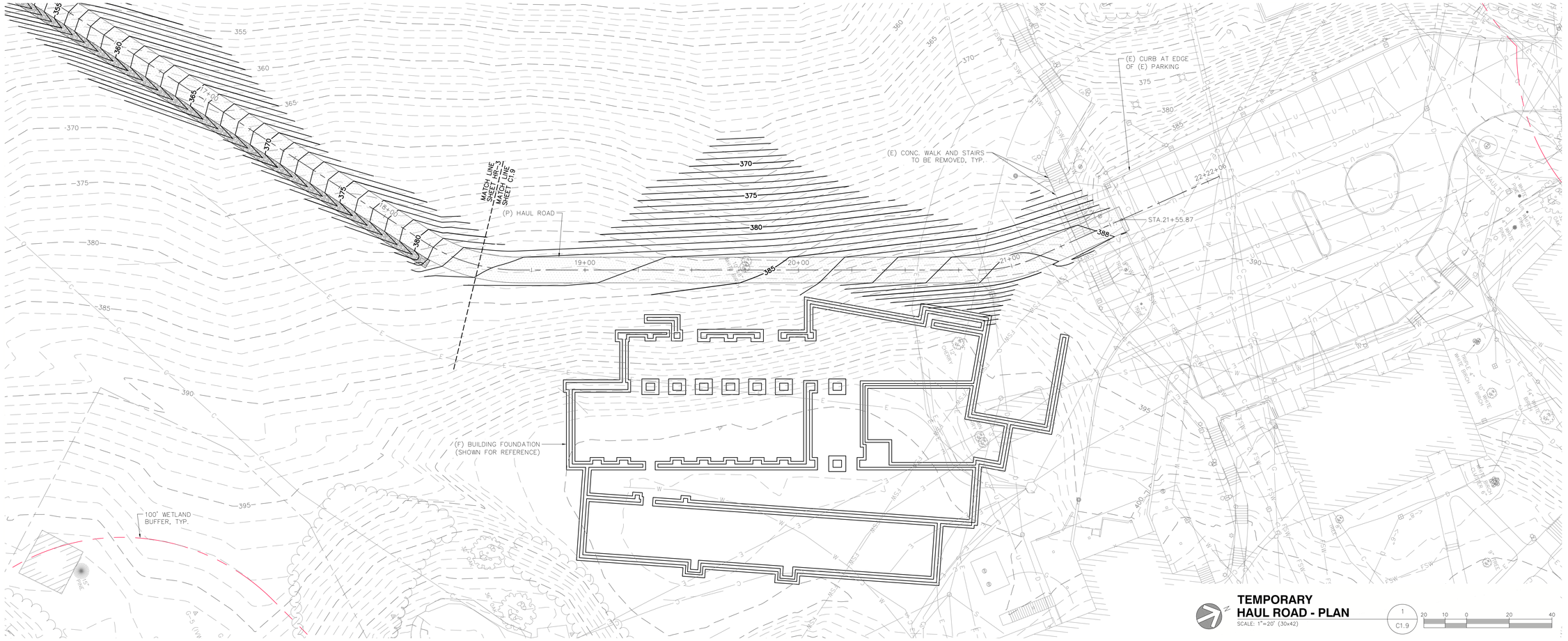
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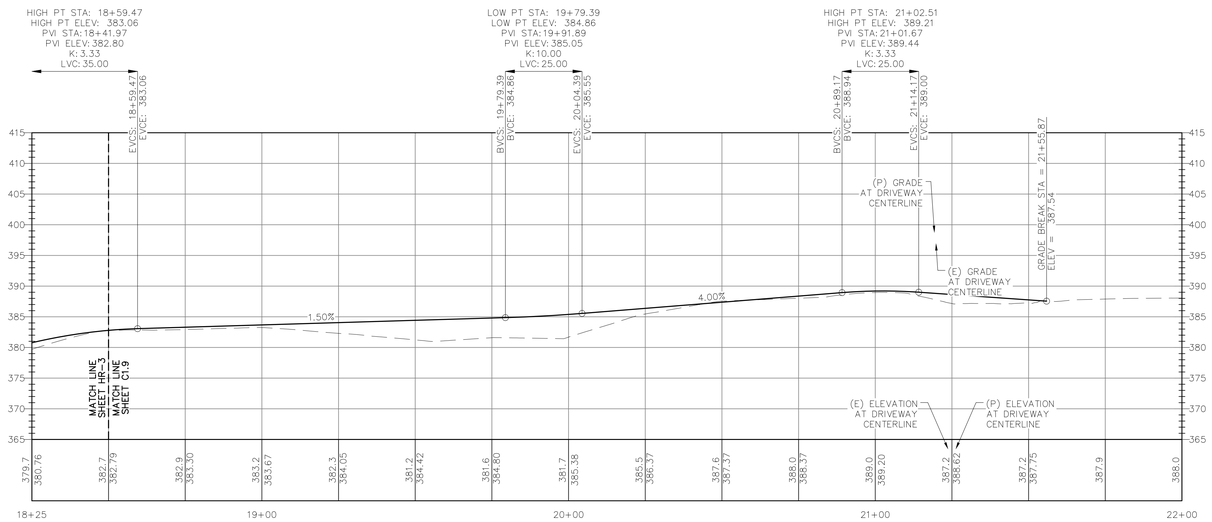
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10 5 0
20 10 0

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WINDIGO
ARCHITECTURE, LLC

914 Mt. Kemble Avenue
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p 973.425.7680
f 973.425.3921

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SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC

4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035

Ph: (413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47916
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

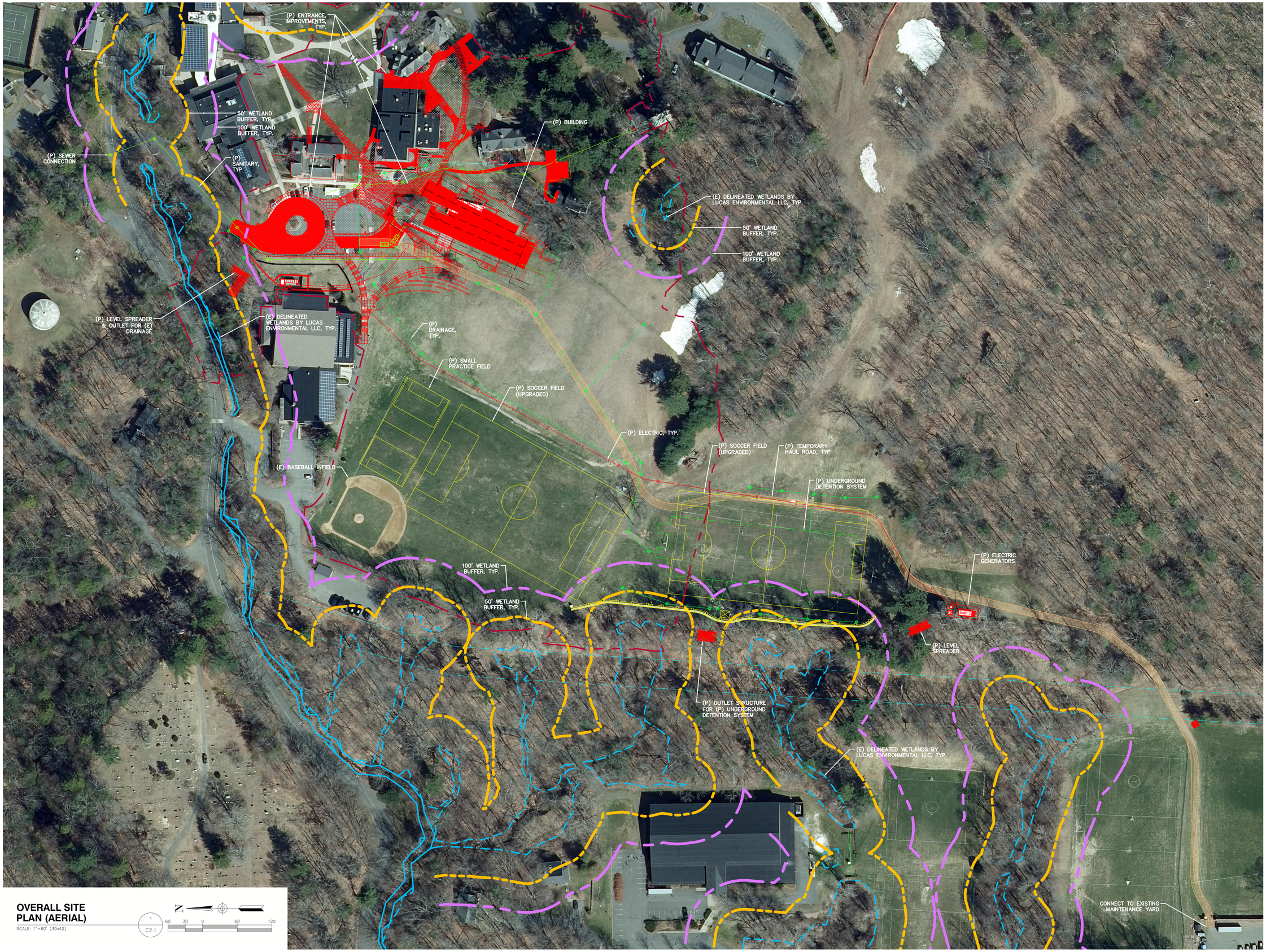
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
TEMPORARY HAUL ROAD PLAN & PROFILE

DRAWING NUMBER
C1.9

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OVERALL SITE PLAN (AERIAL)

SCALE: 1"=60' (30x42)

1 / C2.1

60 30 0 60 120

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ARCHITECTURE, LLC

914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

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	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47915
REGISTERED
PROFESSIONAL ENGINEER
3-8-2023

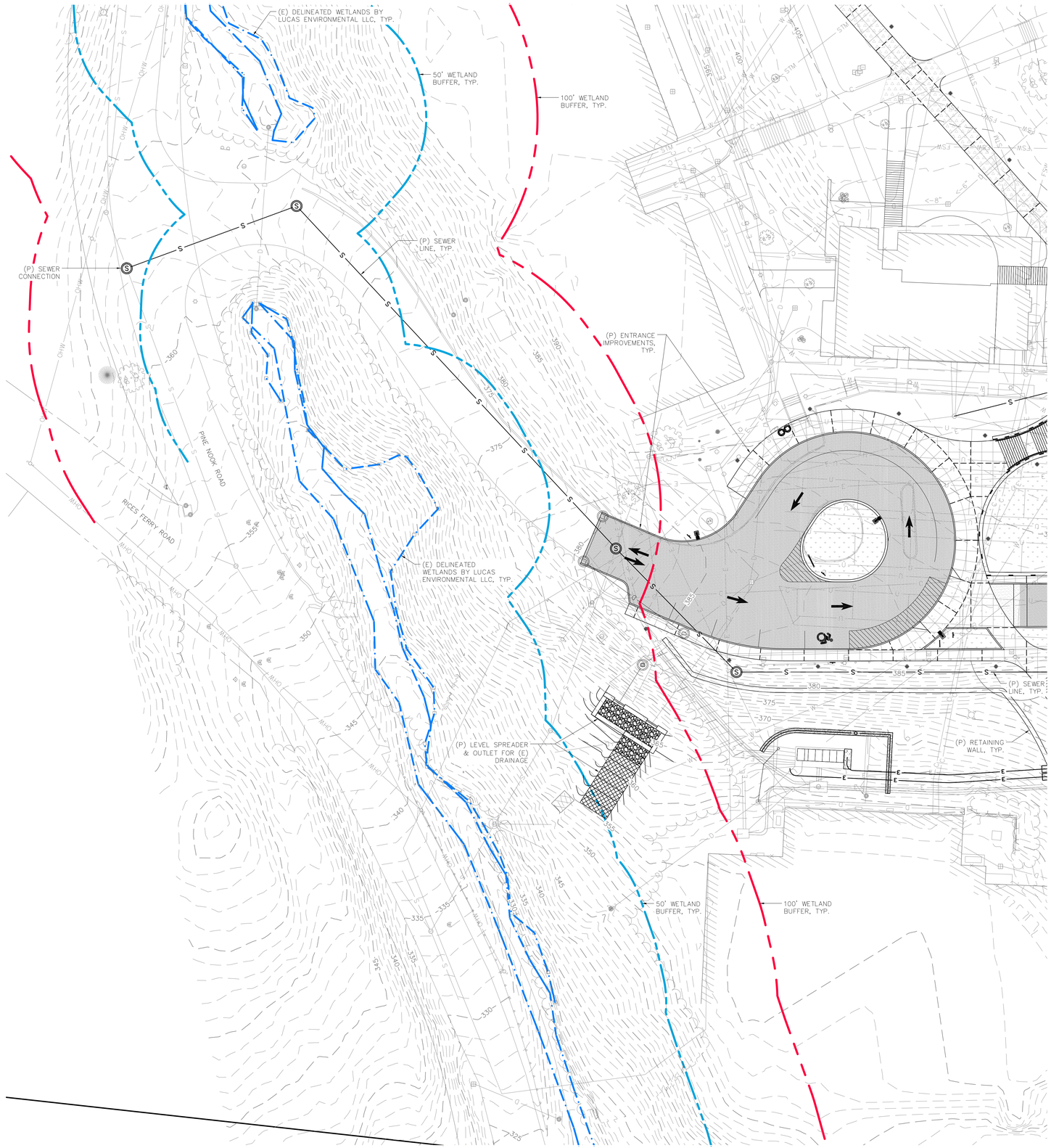
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
OVERALL SIT E
PLAN (AERIAL)

DRAWING NUMBER
C2.1

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DETAIL SITE PLAN

SCALE: 1"=20' (30x42)

1 / C2.2

20 10 0 20 40

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	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAIL SITE PLAN

DRAWING NUMBER
C2.2



DETAIL SITE PLAN

SCALE: 1"=30' (30x42)

1 / C2.3

30 15 0 30 60

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Morristown, NJ 07960
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	03/08/23	NOI SUBMITTAL

PROGRESS SET
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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
No. 47815
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAIL SITE PLAN

DRAWING NUMBER
C2.3



GRADING PLAN

SCALE: 1"=30' (30x42)

1 / C3.1

30 15 0 30 60

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Morristown, NJ 07960
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PROGRESS SET
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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
No. 47816
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

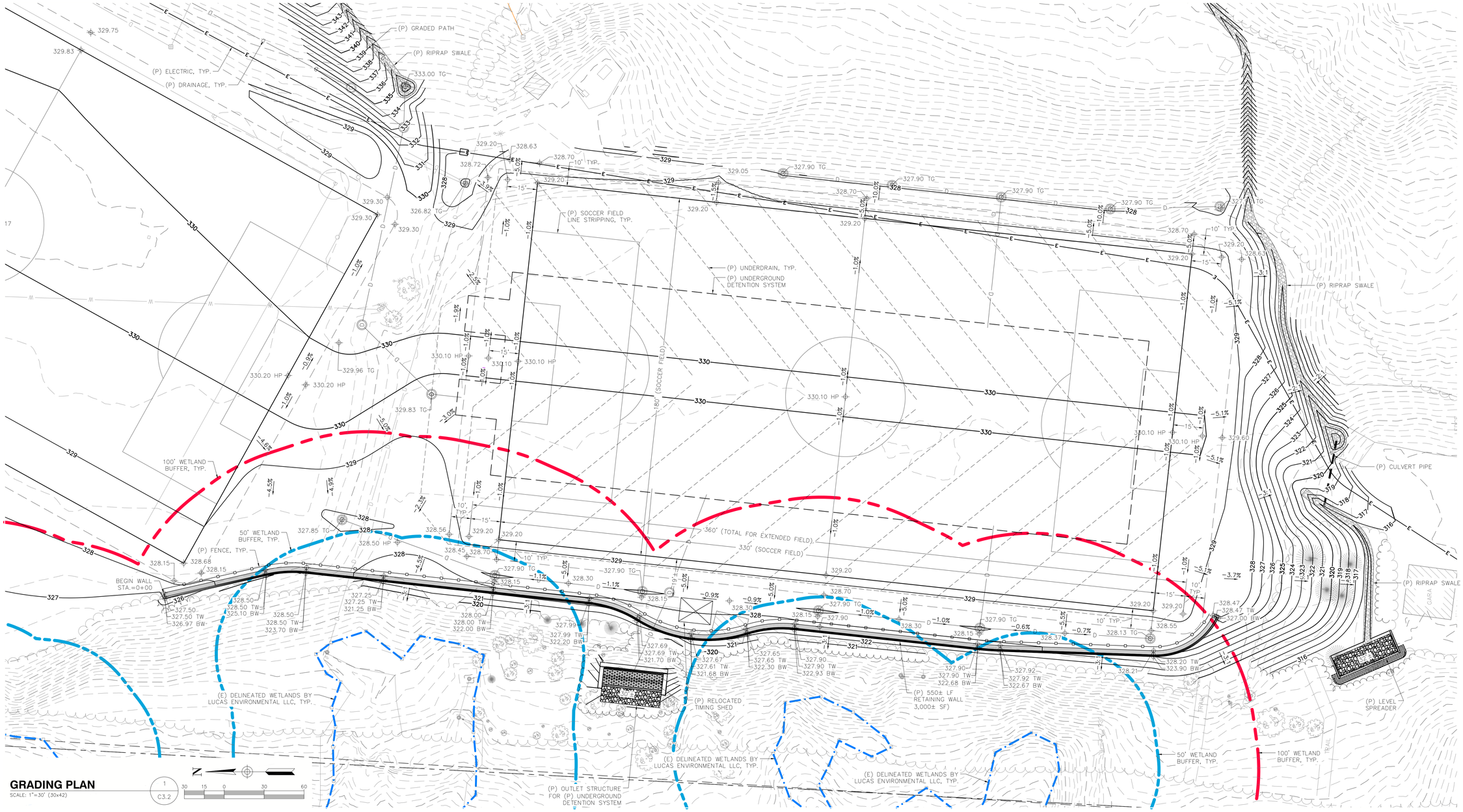
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
GRADING PLAN

DRAWING NUMBER
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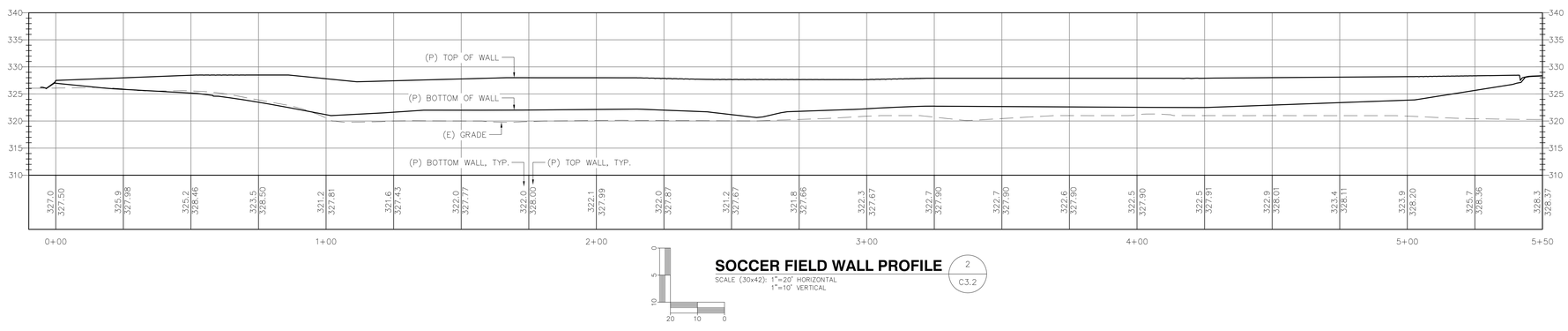


GRADING PLAN

SCALE: 1"=30' (30x42)

1 / C3.2

30 15 0 30 60



SOCCER FIELD WALL PROFILE

SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL

2 / C3.2

20 10 0

0 5 10

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914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

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DESIGN GROUP, LLC

4 Bay Road, Bldg A
Suite 200
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	03/08/23	NOI SUBMITTAL

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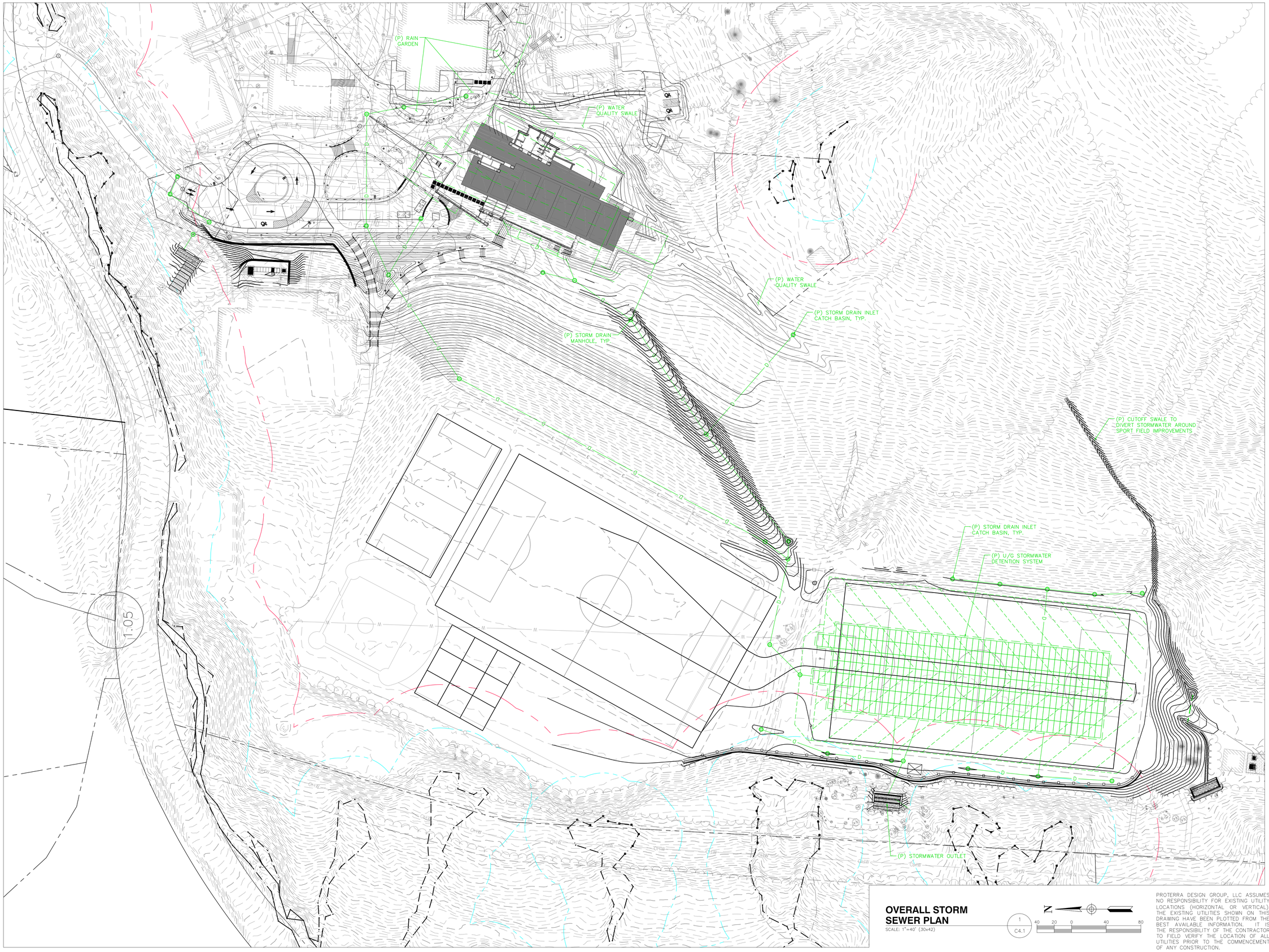
COMMONWEALTH OF MASSACHUSETTS
JESSE M. NIVIENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
GRADING PLAN

DRAWING NUMBER
C3.2



OVERALL STORM SEWER PLAN
SCALE: 1"=40' (30x42)

1 / C4.1

40 20 0 40 80

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p 973.425.7680
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	03/08/23	NOI SUBMITTAL

PROGRESS SET
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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47916
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

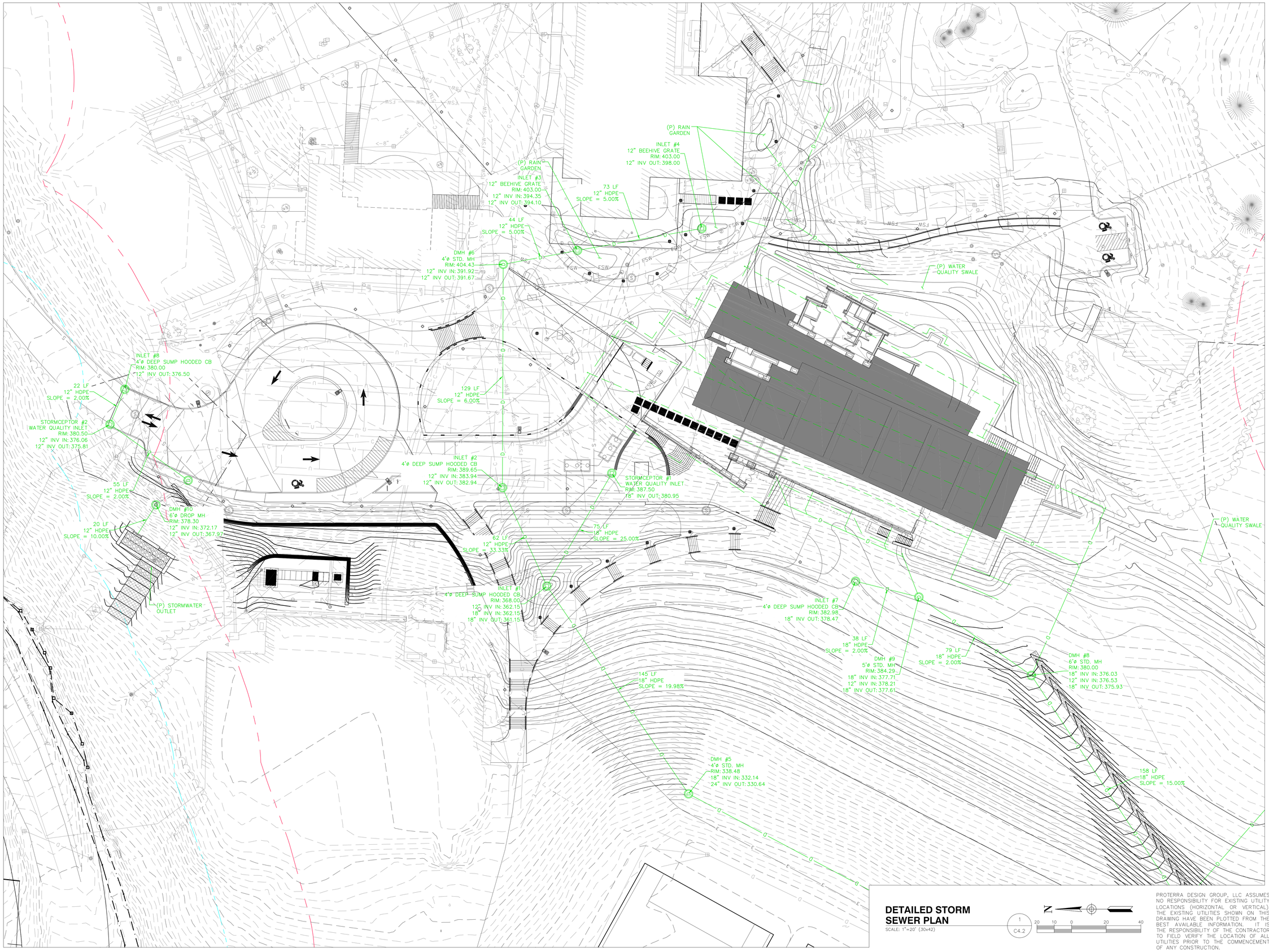
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
OVERALL STORM SEWER PLAN

DRAWING NUMBER
C4.1

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DETAILED STORM SEWER PLAN

SCALE: 1"=20' (30x42)

1 / C4.2

20 10 0 20 40

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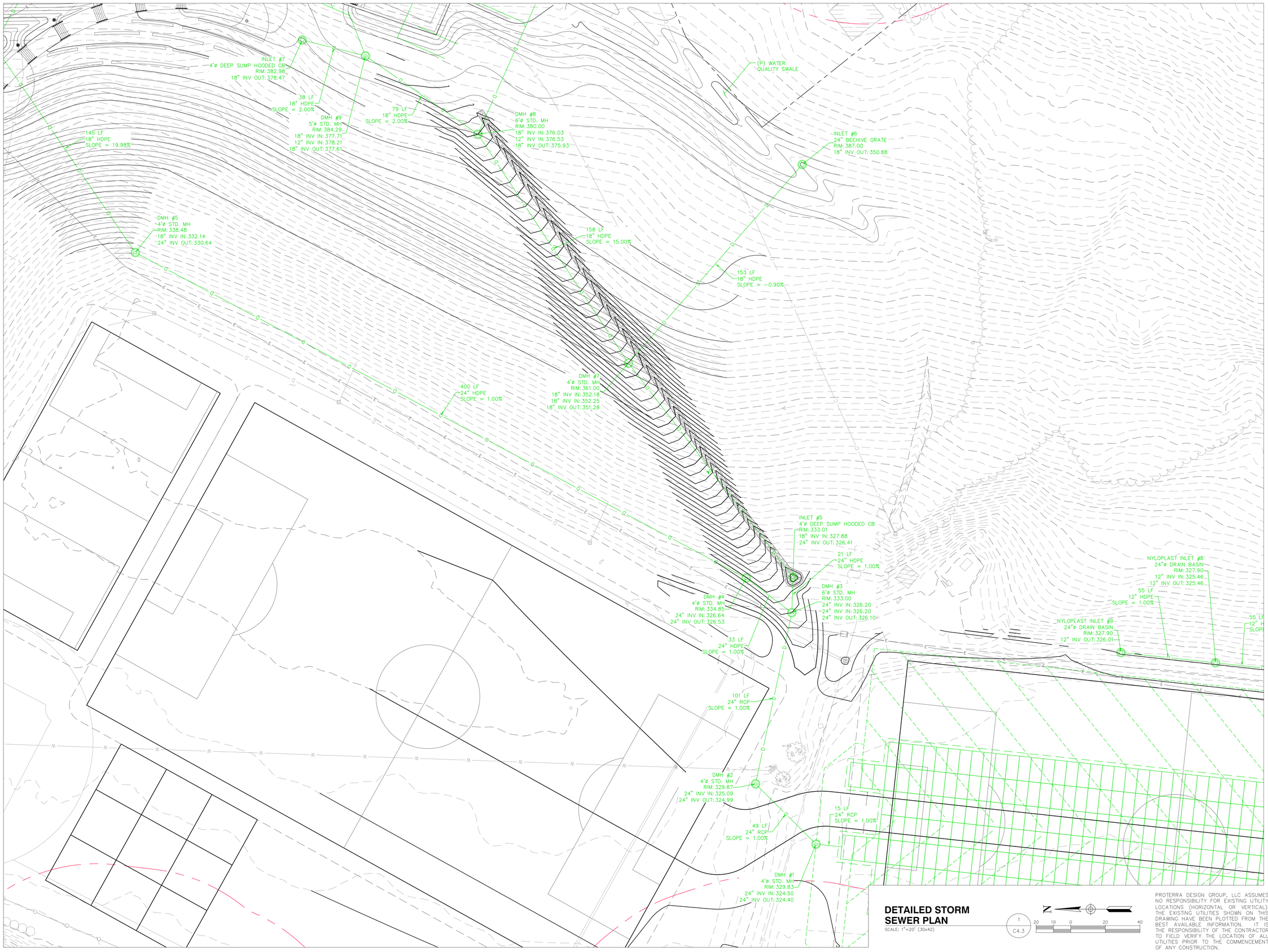
COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAILED STORM SEWER PLAN

DRAWING NUMBER
C4.2



DETAILED STORM SEWER PLAN

SCALE: 1"=20' (30x42)

1 / C4.3

20 10 0 20 40

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	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. HOVENO
CIVIL
No. 47916
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

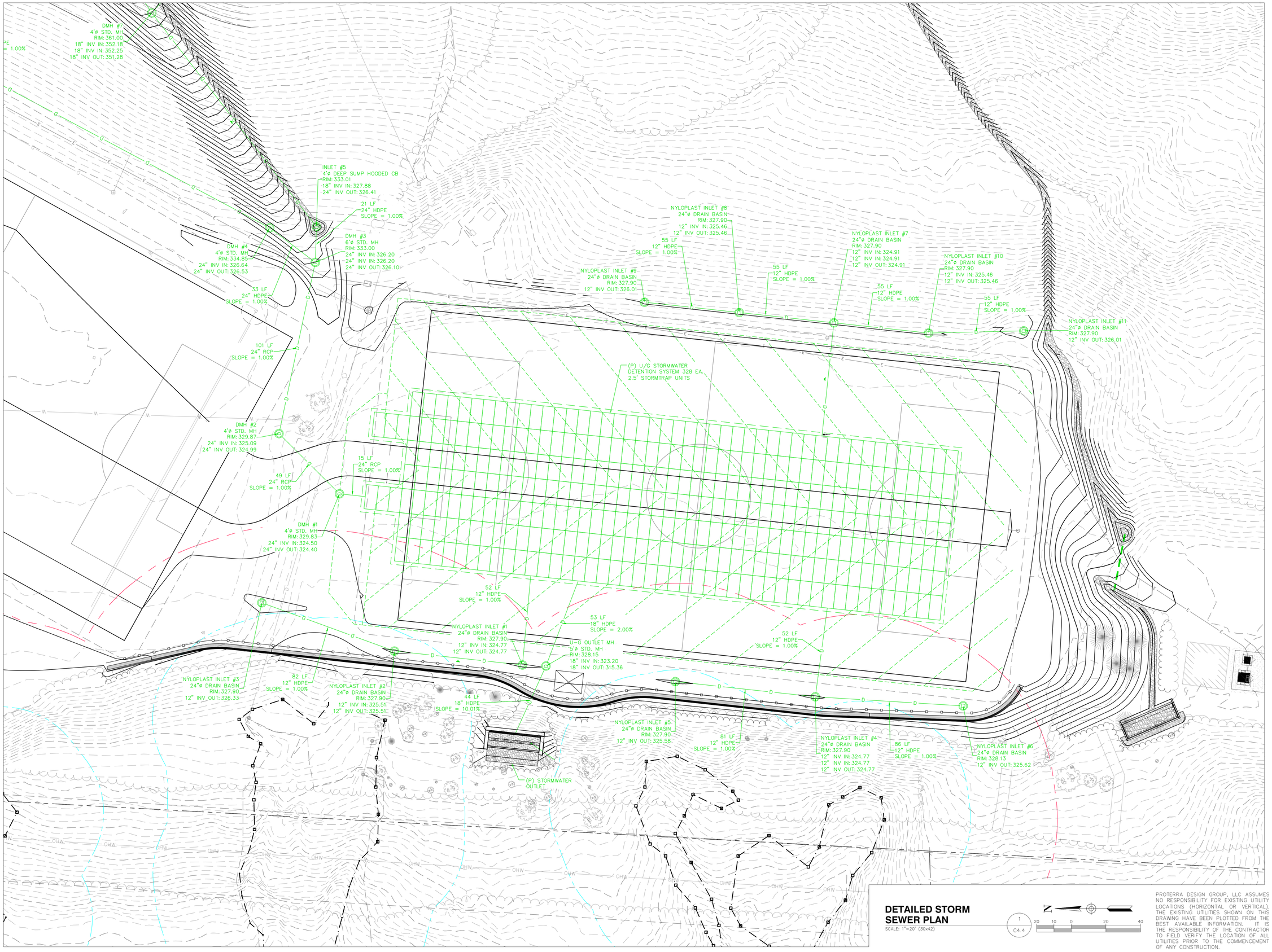
PROJECT NUMBER
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SCALE
SEE PLAN

DRAWING NAME
DETAILED STORM SEWER PLAN

DRAWING NUMBER
C4.3

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DETAILED STORM SEWER PLAN

SCALE: 1"=20' (30x42)

1 / C4.4

20 10 0 20 40

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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47815
REGISTERED PROFESSIONAL ENGINEER
3-6-2023

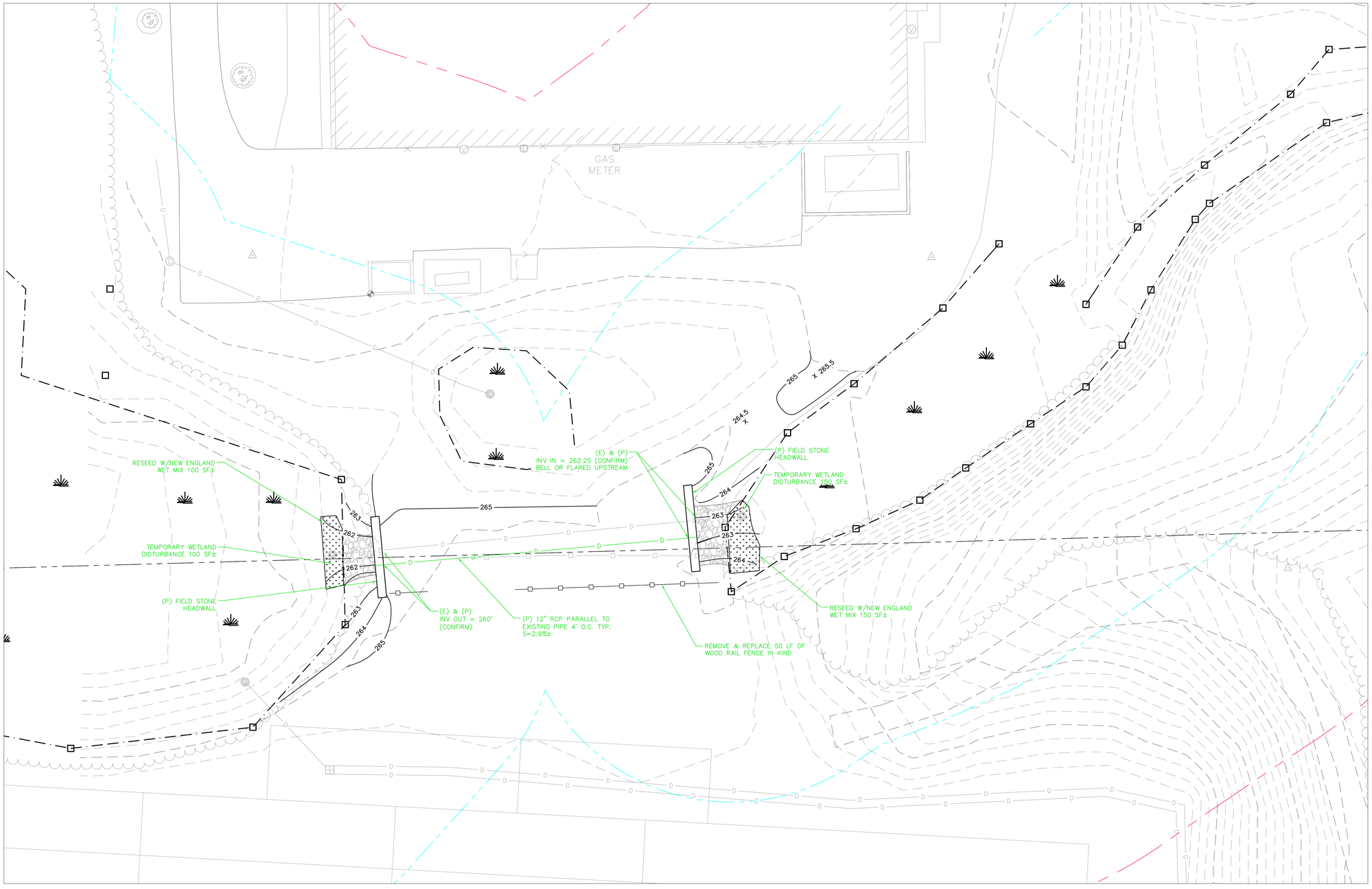
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAILED STORM SEWER PLAN

DRAWING NUMBER
C4.4

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TEMPORARY WETLAND DISTURBANCE

2"–3" ROUND RIVERSTONE OVER FILTER FABRIC

DETAILED STORM SEWER UPGRADE PLAN

SCALE: 1"=10' (30x42)

1 / C4.5

10 5 0 10 20

N

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	03/08/23	NOI SUBMITTAL

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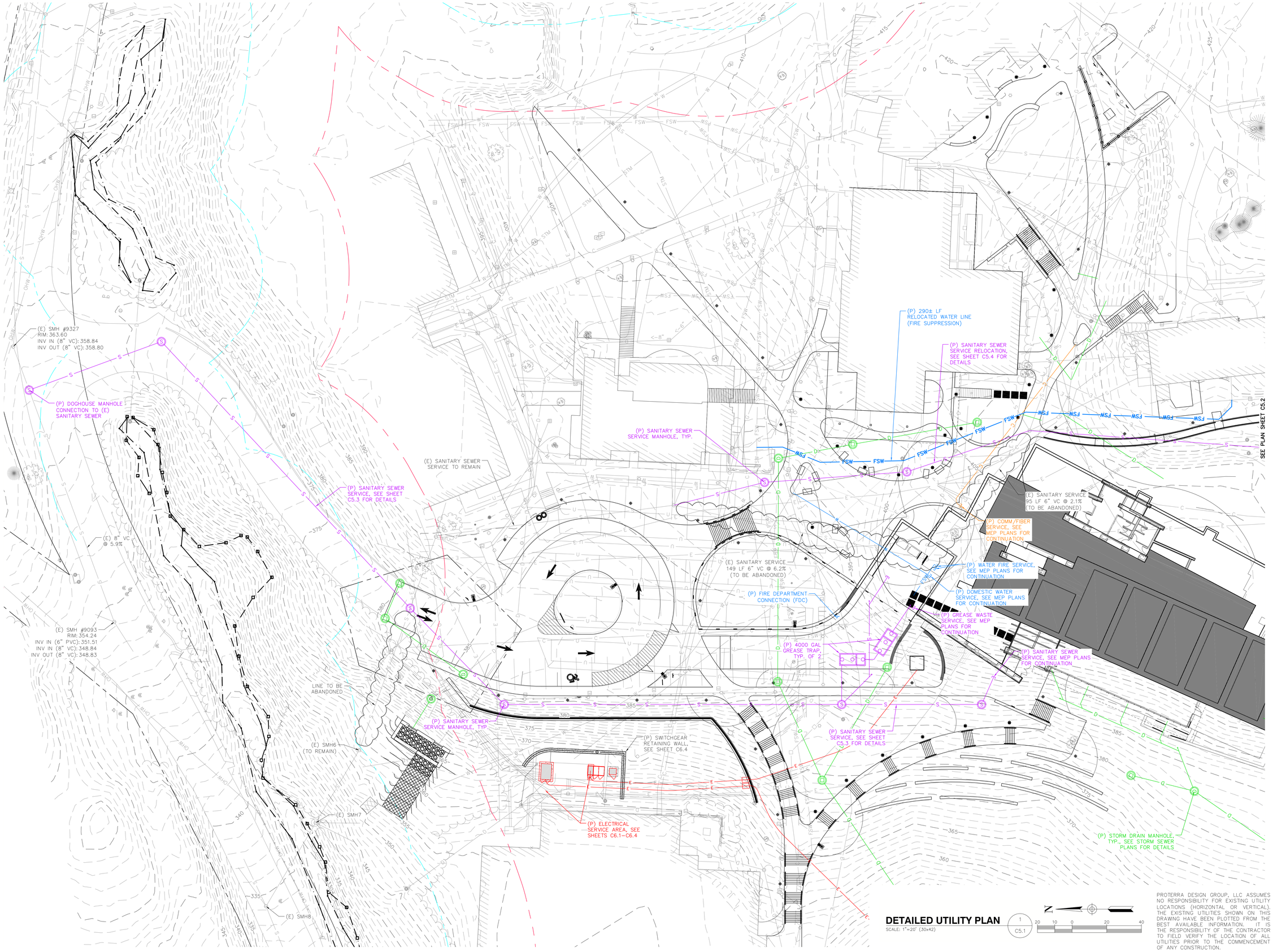
COMMONWEALTH OF MASSACHUSETTS
JESSE M. ANDREHO
CIVIL
No. 47218
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAILED STORM SEWER UPGRADE PLAN

DRAWING NUMBER
C4.5



DETAILED UTILITY PLAN

SCALE: 1"=20' (30x42)

1 / C5.1

20 10 0 20 40

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ISSUE

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	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47315
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
DETAILED UTILITY PLAN

DRAWING NUMBER
C5.1

WINDIGO
ARCHITECTURE, LLC

914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC

4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035

Ph: (413)320-4918

(P) 290± LF
RELOCATED WATER LINE
(FIRE SUPPRESSION)

(P) SANITARY SEWER
SERVICE RELOCATION,
SEE SHEET C5.4 FOR
DETAILS

(P) SANITARY SEWER
SERVICE MANHOLE, TYP.

(E) SANITARY SEWER
SERVICE TO REMAIN

(E) SANITARY SERVICE
149 LF 6" VC @ 6.2%
(TO BE ABANDONED)

(P) FIRE DEPARTMENT
CONNECTION (FDC)

SEE PLAN SHEET C5.1

(P) 4000 GAL
GREASE TRAP,
TYP. OF 2

(P) SWITCHGEAR
RETAINING WALL,
SEE SHEET C6.4

(P) ELECTRICAL
SERVICE AREA, SEE
SHEETS C6.1–C6.4

(E) SANITARY SERVICE
95 LF 6" VC @ 2.1%
(TO BE ABANDONED)

(P) COMM/FIBER
SERVICE, SEE
MEP PLANS FOR
CONTINUATION

(P) WATER FIRE SERVICE,
SEE MEP PLANS FOR
CONTINUATION

(P) DOMESTIC WATER
SERVICE, SEE MEP PLANS
FOR CONTINUATION

(P) GREASE WASTE
SERVICE, SEE MEP
PLANS FOR
CONTINUATION

(P) SANITARY SEWER
SERVICE, SEE SHEET
C5.3 FOR DETAILS

(P) SANITARY SEWER
SERVICE MANHOLE, TYP.

(P) SANITARY SEWER
SERVICE, SEE MEP PLANS
FOR CONTINUATION

(P) NATURAL GAS
SERVICE, SEE MEP
PLANS FOR
CONTINUATION

CONNECT TO EX. STUB,
VERIFY EX. CAPACITY FOR
NATURAL GAS CONNECTION

(P) SANITARY SEWER
SERVICE, SEE SHEET
C5.5 FOR DETAILS

(P) SANITARY SEWER
SERVICE, SEE SHEET
C5.5 FOR DETAILS

(P) STORM DRAIN MANHOLE,
TYP., SEE STORM SEWER
PLANS FOR DETAILS

DETAILED UTILITY PLAN

SCALE: 1"=20' (30x42)

1
C5.2

20 10 0 20 40

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Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

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	03/08/23	NOI SUBMITTAL

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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47315
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

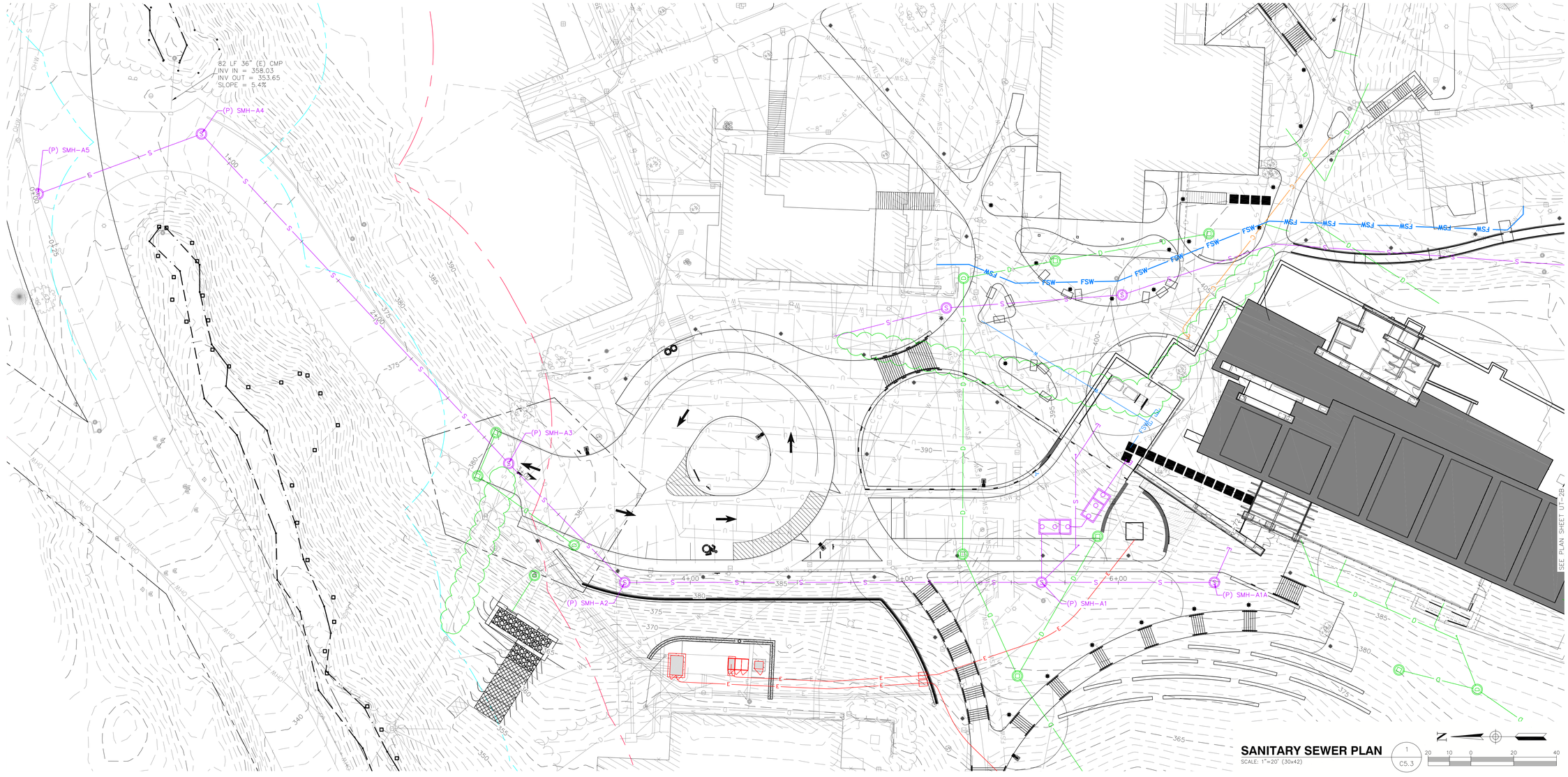
PROJECT NUMBER
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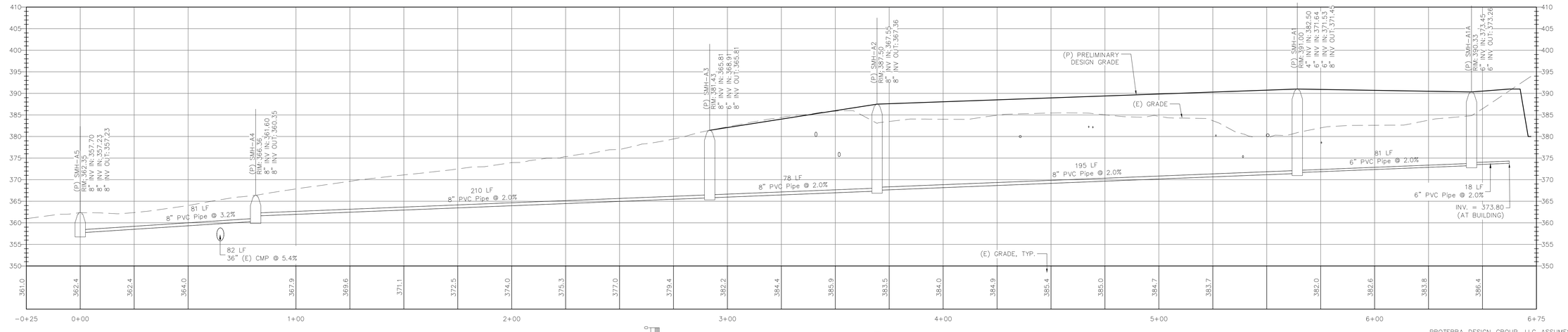
SCALE
SEE PLAN

DRAWING NAME
DETAILED UTILITY PLAN

DRAWING NUMBER
C5.2

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SANITARY SEWER PROFILE
SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL
2
C5.3

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PROGRESS SET
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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47815
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SANITARY SEWER
PLAN & PROFILE

DRAWING NUMBER
C5.3

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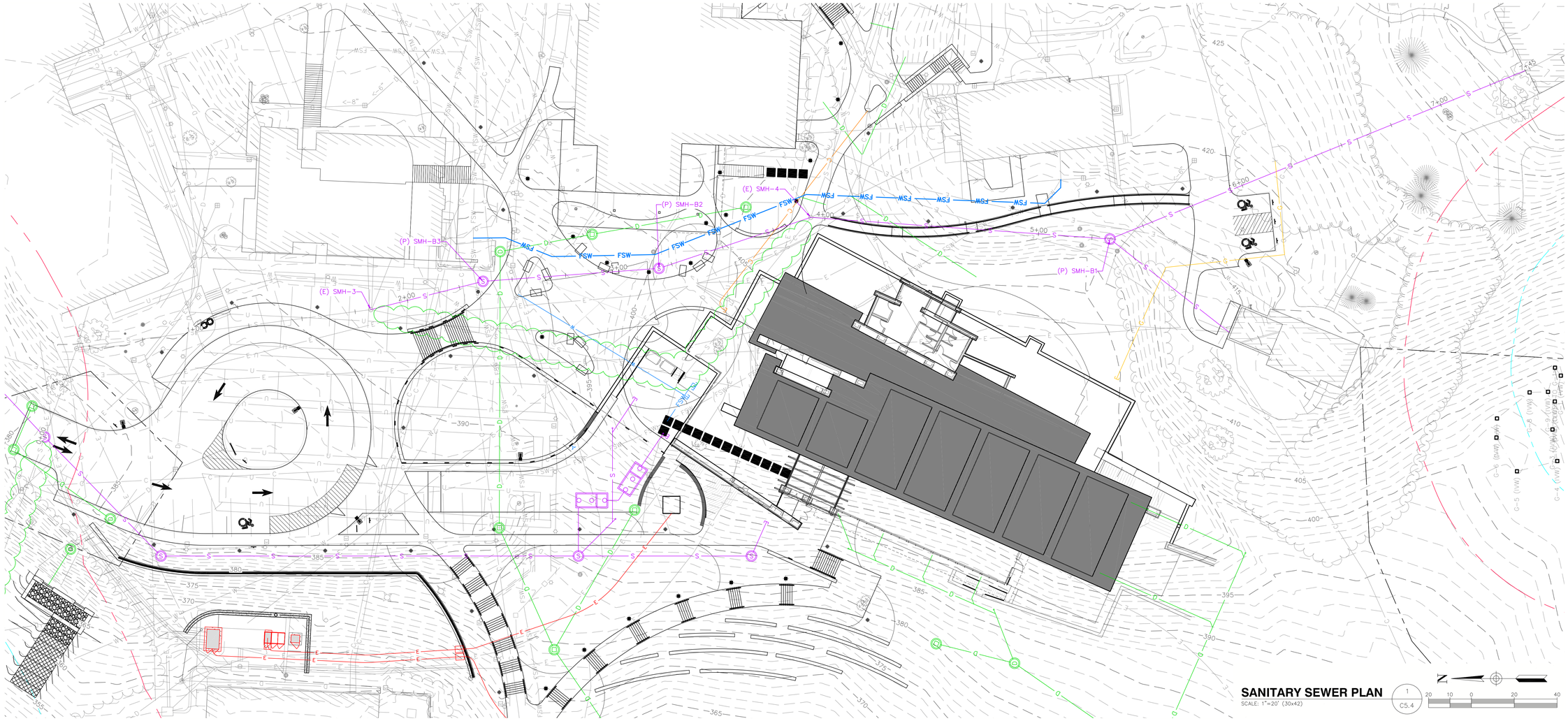
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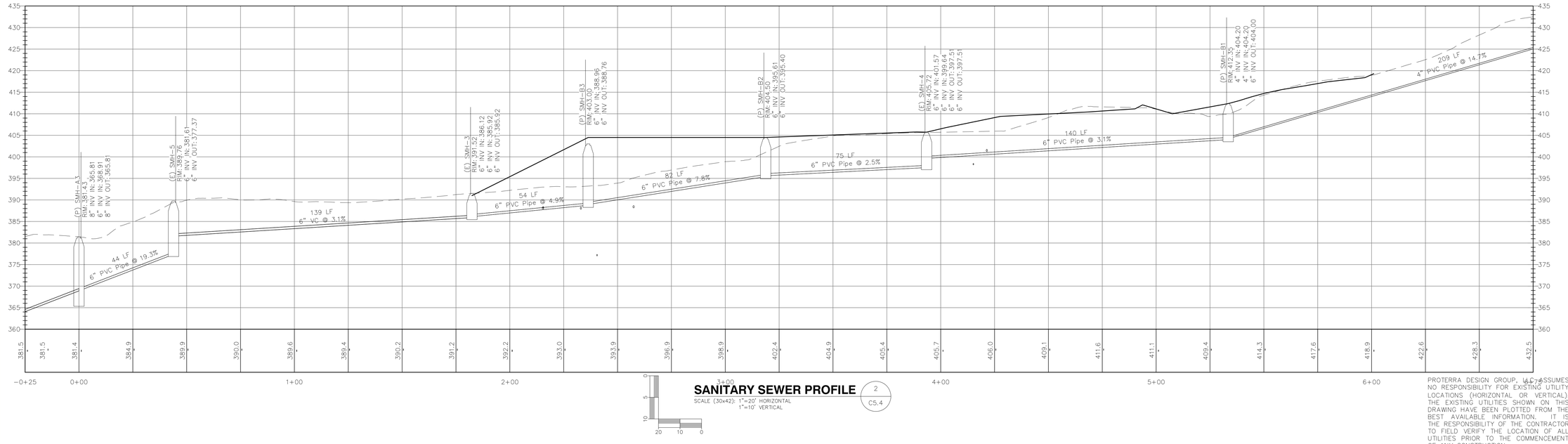


SANITARY SEWER PLAN

SCALE: 1"=20' (30x42)

1 / C5.4

20 10 0 20 40



SANITARY SEWER PROFILE

SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL

2 / C5.4

0 5 10
20 10 0

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COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47219
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

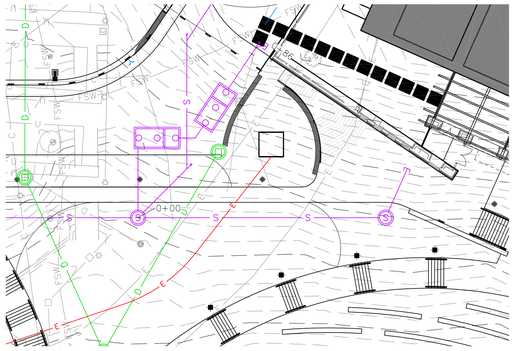
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SANITARY SEWER PLAN & PROFILE

DRAWING NUMBER
C5.4

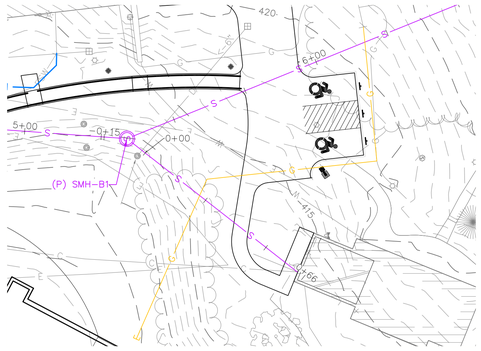
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GREASE TRAP SANITARY SEWER PLAN

SCALE: 1"=20' (30x42)

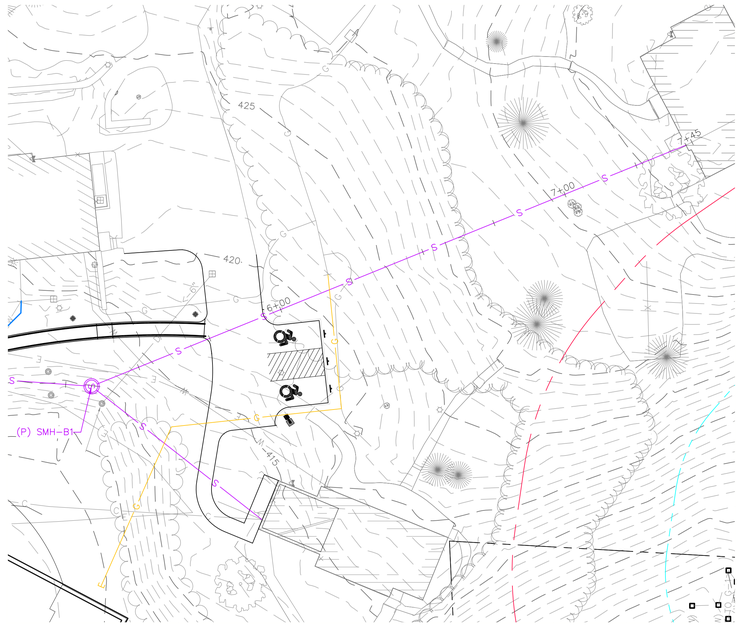
1 / C5.5



SWIFT HOUSE SANITARY SEWER SERVICE PLAN

SCALE: 1"=20' (30x42)

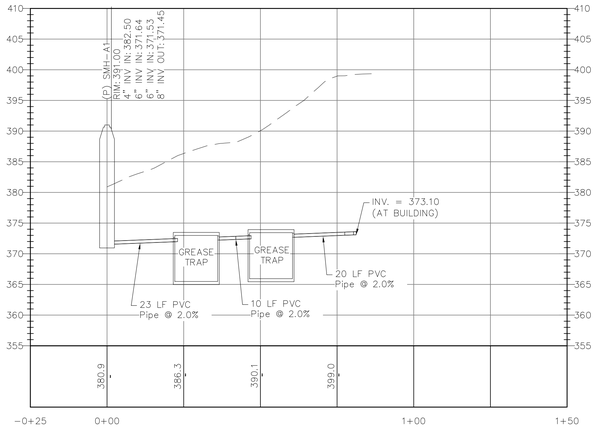
3 / C5.5



LITTLE HUNDRIDGE SANITARY SEWER SERVICE PLAN

SCALE: 1"=20' (30x42)

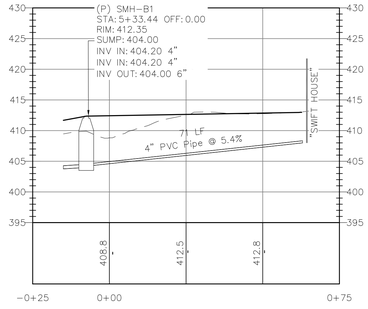
5 / C5.5



GREASE TRAP SANITARY SEWER PROFILE

SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL

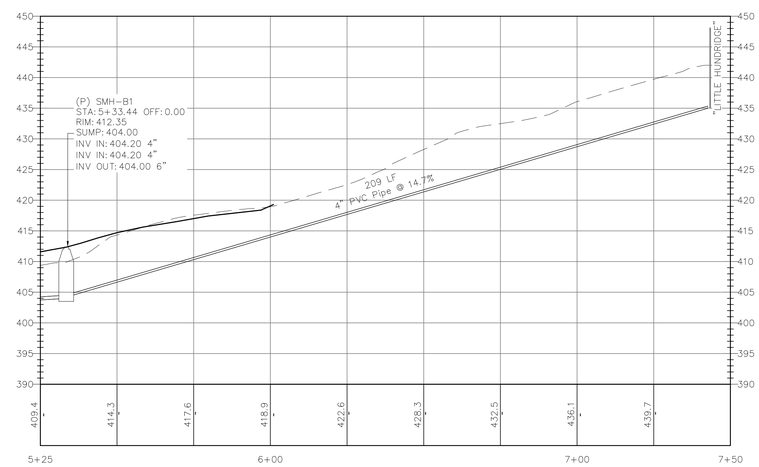
2 / C5.5



SWIFT HOUSE SANITARY SEWER SERVICE PROFILE

SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL

4 / C5.5



LITTLE HUNDRIDGE SANITARY SEWER SERVICE PROFILE

SCALE (30x42): 1"=20' HORIZONTAL
1"=10' VERTICAL

6 / C5.5

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ARCHITECTURE, LLC

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Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

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SITE CONSULTANT

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DESIGN GROUP, LLC

4 Bay Road, Bldg A
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Ph: (413)320-4918

Proposed Dining Hall
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PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SANITARY SEWER PLAN & PROFILE

DRAWING NUMBER
C5.5



GENERATOR CONDUIT PLAN (AERIAL)

SCALE: 1"=20' (30x42)

1 / C6.1

40 20 0 40 40

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COMMONWEALTH OF MASSACHUSETTS
JESSE M. HINDRO
No. 47915
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SWITCHGEAR OVERALL PLAN

DRAWING NUMBER
C6.1

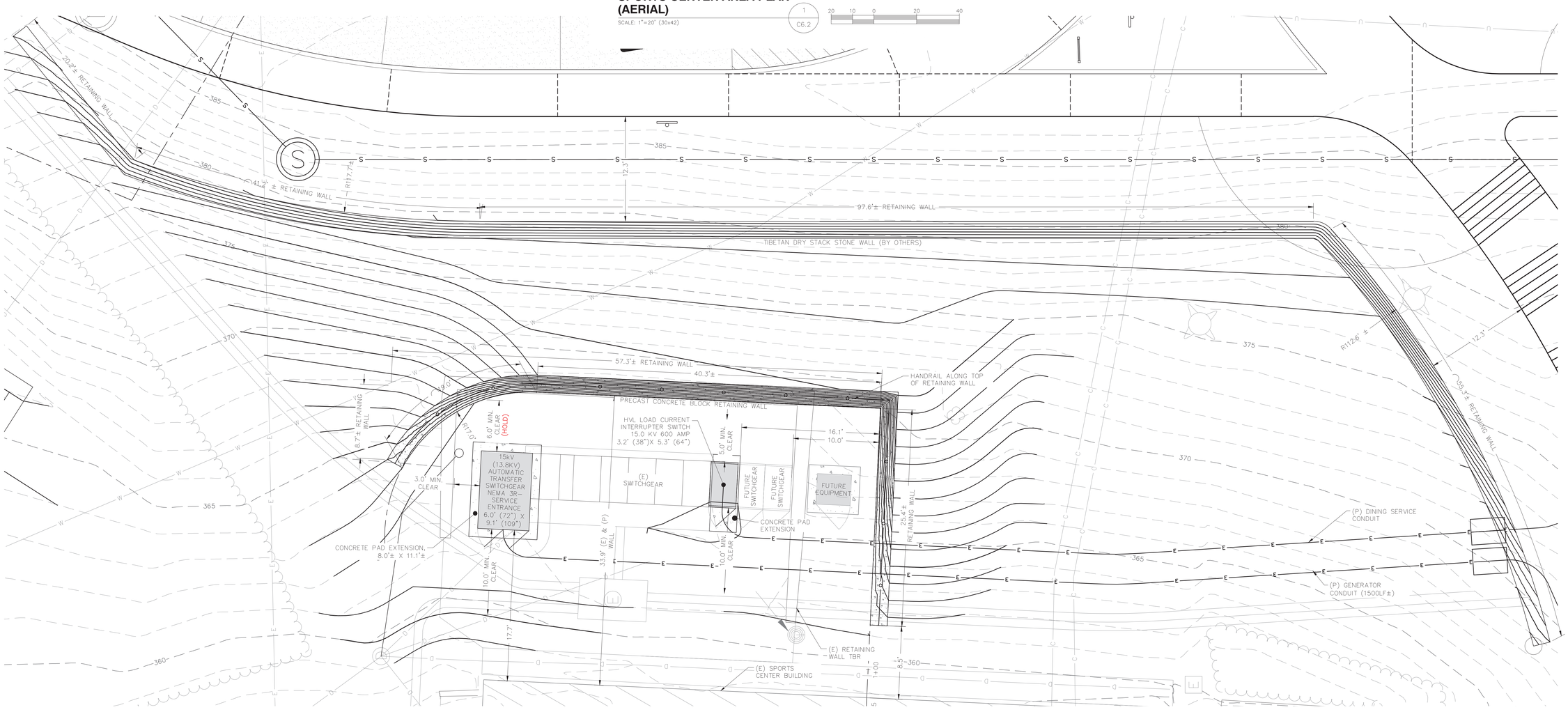


SPORTS CENTER AREA PLAN (AERIAL)

SCALE: 1"=20' (30x42)

1 / C6.2

20 10 0 20 40



ELECTRICAL SERVICE AREA PLAN

SCALE: 1"=5' (30x42)

2 / C6.2

5 2.5 0 5 10

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PROGRESS SET
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JESSE M. HOPKINS
No. 47918
REGISTERED PROFESSIONAL ENGINEER
COMMONWEALTH OF MASSACHUSETTS
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SWITCHGEAR DETAIL PLAN

DRAWING NUMBER
C6.2

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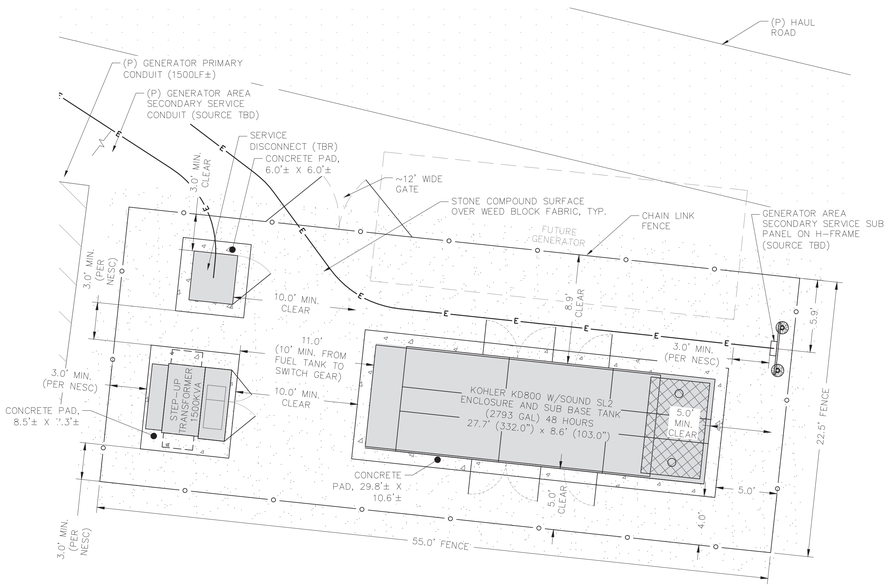


MAINTENANCE/STORAGE YARD PLAN (AERIAL)

SCALE: 1"=20' (30x42)

1 / C6.3

20 10 0 20 40

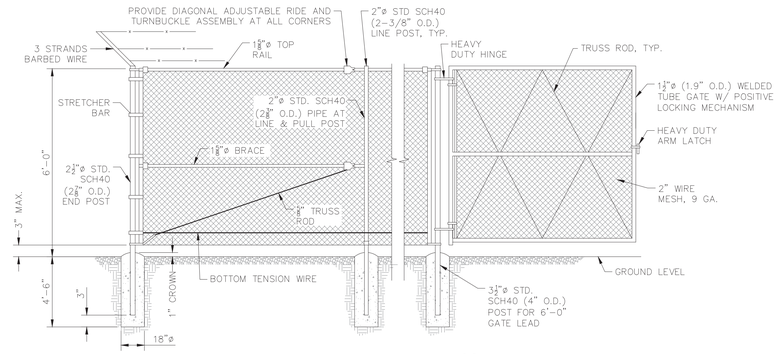


REMOTE GENERAOR AREA PLAN

SCALE: 1"=5' (30x42)

2 / C6.3

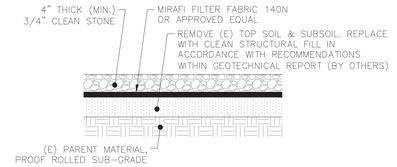
5 2.5 0 5 10



CHAIN LINK FENCE

SCALE: NONE

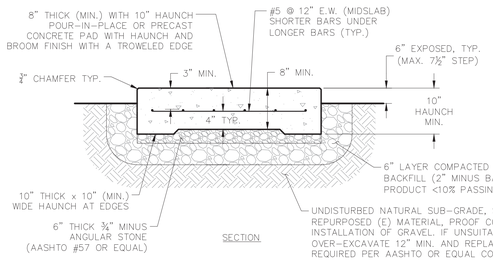
3 / C6.3

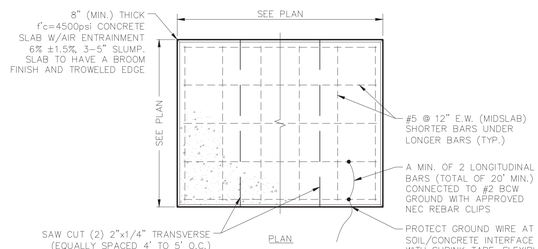


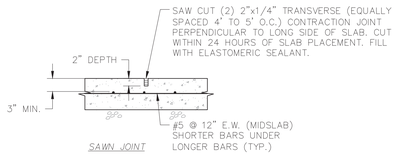
COMPOUND SURFACE

SCALE: NONE

4 / C6.3







CONCRETE PAD DETAIL

SCALE: NONE

5 / C6.3

CONCRETE NOTES:

1. BEARING STRATA MEDIUM TO DENSE INSET GRANULAR MATERIAL OR COMPACTED FILL. 95% COMPACTION.
2. SUBGRADE AND FILL SHALL CONSIST OF CLEAN SOIL. NO DELETERIOUS MATERIALS OR ORGANICS TO BE USED.
3. CONCRETE FORM WORK SHALL BE CONSTRUCTED IN WORKMAN LIKE MANNER. STRIP AND REMOVE UPON COMPLETION.
4. CONCRETE SHALL HAVE 4500PSI 28-DAY COMPRESSIVE STRENGTH WITH 6% ±1.5% AIR ENTRAINMENT, 4(±1)" SLUMP AND BRISTLE BROOM FINISH AND TROWELED OR CHAMFERED EDGE.
5. SLOPE SURFACE TO DRAIN
6. SEE CONCRETE NOTES ON GN-1.

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PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. PINSKNEY
CIVIL
No. 47915
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

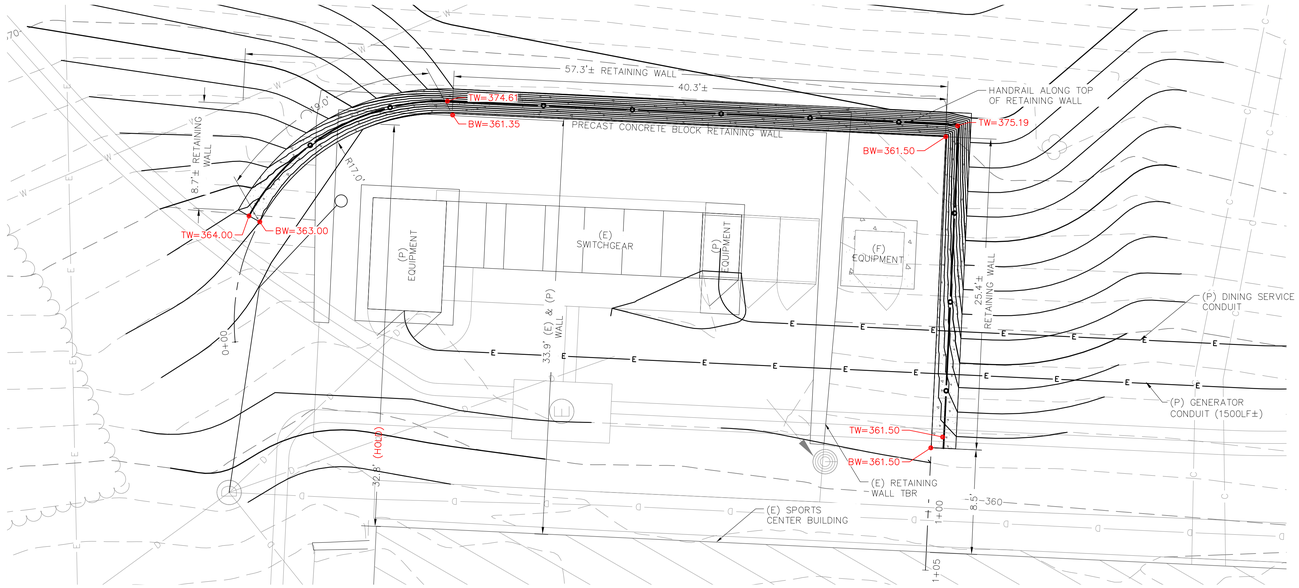
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
BACKUP GENERATOR PLAN

DRAWING NUMBER
C6.3

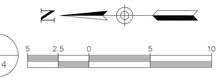
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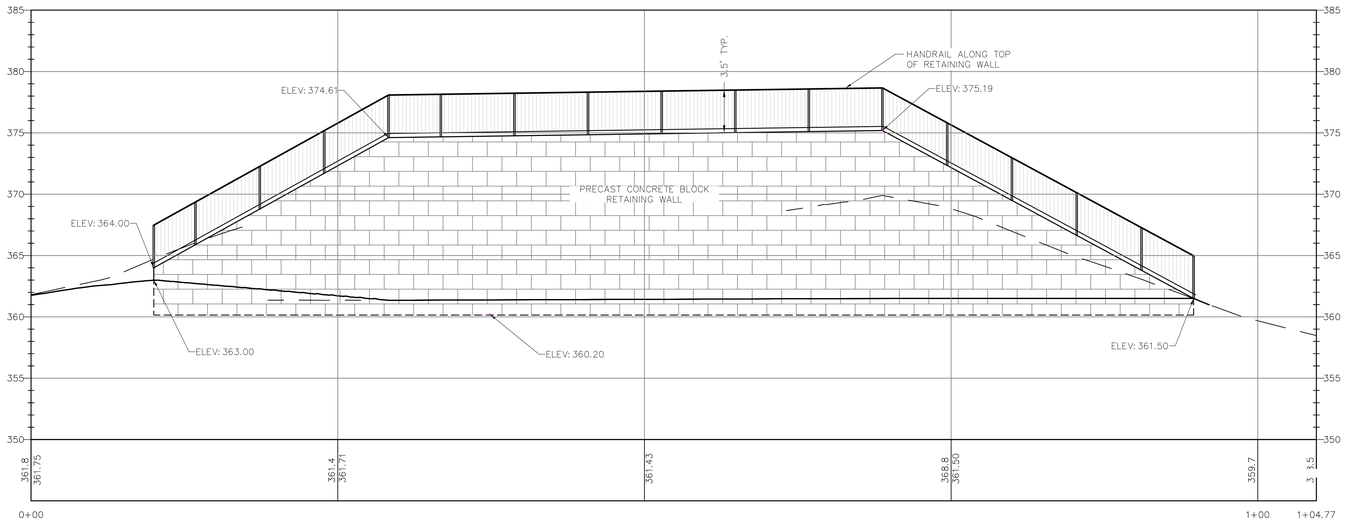


ELECTRICAL SERVICE AREA RETAINING WALL PLAN

1 / C6.4

SCALE: 1"=5' (30x42)

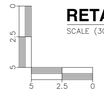


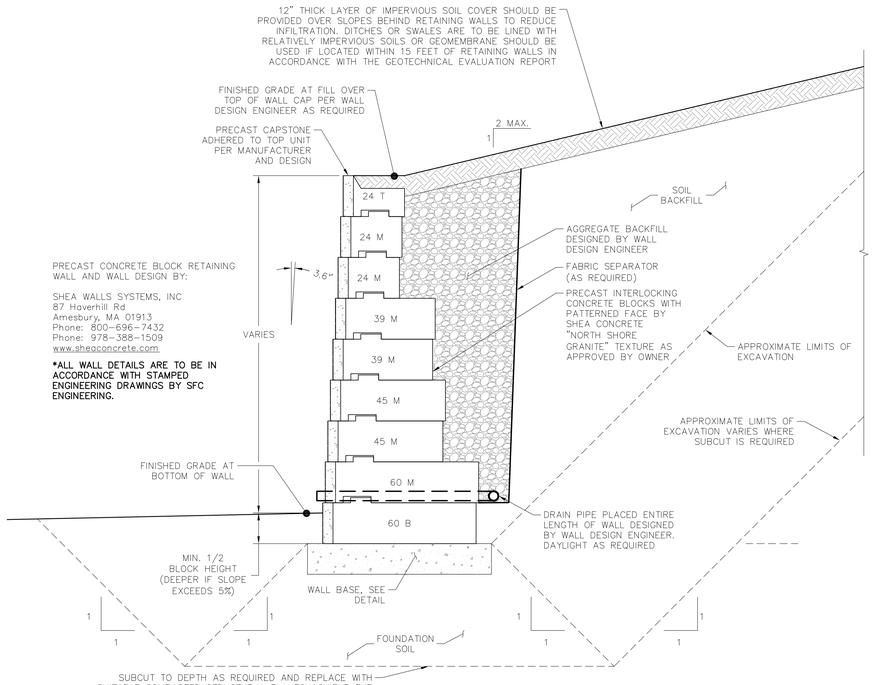


RETAINING WALL PROFILE

2 / C6.4

SCALE (30x42): 1"=5' HORIZONTAL
1"=5' VERTICAL



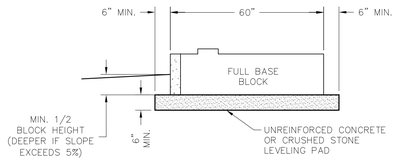


NOTES:
1. THIS DETAIL IS FOR REFERENCE PURPOSES ONLY. ENGINEERED PLANS BY SFC ENGINEERING PROVIDED BY SHEA WALL SYSTEMS INC.
2. DAYLIGHT DRAIN PIPE AT LOW POINTS THROUGH WALL FACE OR AT ENDS. MAXIMUM SPACING 40 FEET OR PER SITE CONDITIONS.

TYPICAL RETAINING WALL SECTION

3 / C6.4

SCALE: NONE



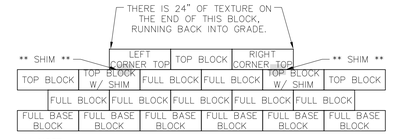
NOTES:
1. THIS DETAIL IS FOR REFERENCE PURPOSES ONLY. ENGINEERED PLANS BY SFC ENGINEERING PROVIDED BY SHEA WALL SYSTEMS INC.
2. LEVELING PAD SHOULD BE AS SPECIFIED BY THE DESIGN ENGINEER IN THE PROJECT PLAN SET.
3. THE WIDTH OF THE LEVELING PAD MUST EXTEND 6" (MINIMUM) IN FRONT AND 6" (MINIMUM) IN BACK OF THE BASE BLOCK. AS A RESULT, THE TYPICAL WIDTH OF LEVELING PAD WOULD BE: 60" DEEP BASE BLOCK...LEVELING PAD WIDTH IS 72"
4. WHEN SETTING THE BASE BLOCK AND CHECKING FOR LEVEL FROM FRONT TO BACK, IT IS RECOMMENDED THAT THE BACK EDGE OF THE BLOCK BE ABOUT 1/2" BELOW THE FRONT EDGE OF THE BLOCK.
5. COMPACTION TO THE SPECIFIED EMBEDMENT DEPTH SHALL BE DONE IN FRONT OF THE BASE BLOCK BEFORE COMPACTION IS DONE BEHIND THE BASE BLOCK. THIS REDUCES THE CHANCE THAT COMPACTION BEHIND THE BASE BLOCK WILL ROLL THE BASE BLOCK FORWARD.
6. SEE BLOCK SPECIFICATION & INSTALLATION INSTRUCTIONS FOR MORE DETAILS.

TYPICAL RETAINING WALL BASE BLOCK DETAIL

4 / C6.4

SCALE: NONE

THE STANDARD PLACEMENT OF A LEFT OR RIGHT CORNER TOP BLOCK IS TO PUT THE FULL 48" LENGTH OF TEXTURE IN THE FACE OF THE WALL AND THE 24" LENGTH OF TEXTURE ON THE END OF THE BLOCK RUNNING BACK INTO GRADE.



** NOTE:
A CONCRETE "SHIM" MUST BE PLACED BETWEEN THE TOP BLOCK AND A LEFT OR RIGHT CORNER TOP BLOCK AT EACH POINT IN THE WALL WHERE THE TOP OF THE WALL STEPS UP. USE A STANDARD CONCRETE MASONRY UNIT (CMU) FOR THE SHIM. THESE ARE GENERALLY AVAILABLE AT A LUMBER YARD, READY MIX PLANT OR MASONRY BLOCK PLANT. THE THICKNESS OF THE "SHIM" SHOULD BE 7 1/2" (YOU MAY NEED TO USE A CHOP SAW TO CUT 1/2" OFF AN 8" CMU). THE SHIM SHOULD BE GLUED TO BOTH THE TOP BLOCK ON WHICH IT IS PLACED AND ALSO TO THE LEFT OR RIGHT CORNER TOP BLOCK THAT IS PLACED ON TOP OF THE SHIM (PL PREMIUM IS THE RECOMMENDED CONCRETE ADHESIVE). ANY REMAINING VOID BETWEEN THE TOP BLOCK AND THE RIGHT CORNER TOP BLOCK SHOULD BE FILLED WITH CRUSHED STONE AND SILTS.

TYPICAL RETAINING WALL TOP OF WALL STEP UP DETAIL

6 / C6.4

SCALE: NONE

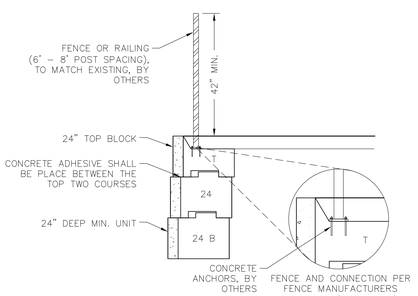
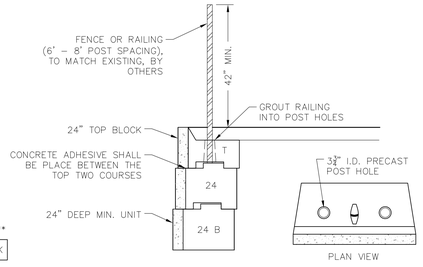


PLATE AND ANCHOR BOLTS



PRECAST FENCE POST HOLES

TYPICAL RETAINING WALL HANDRAIL DETAIL

5 / C6.4

SCALE: NONE

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WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
Ph:(413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
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	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47215
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

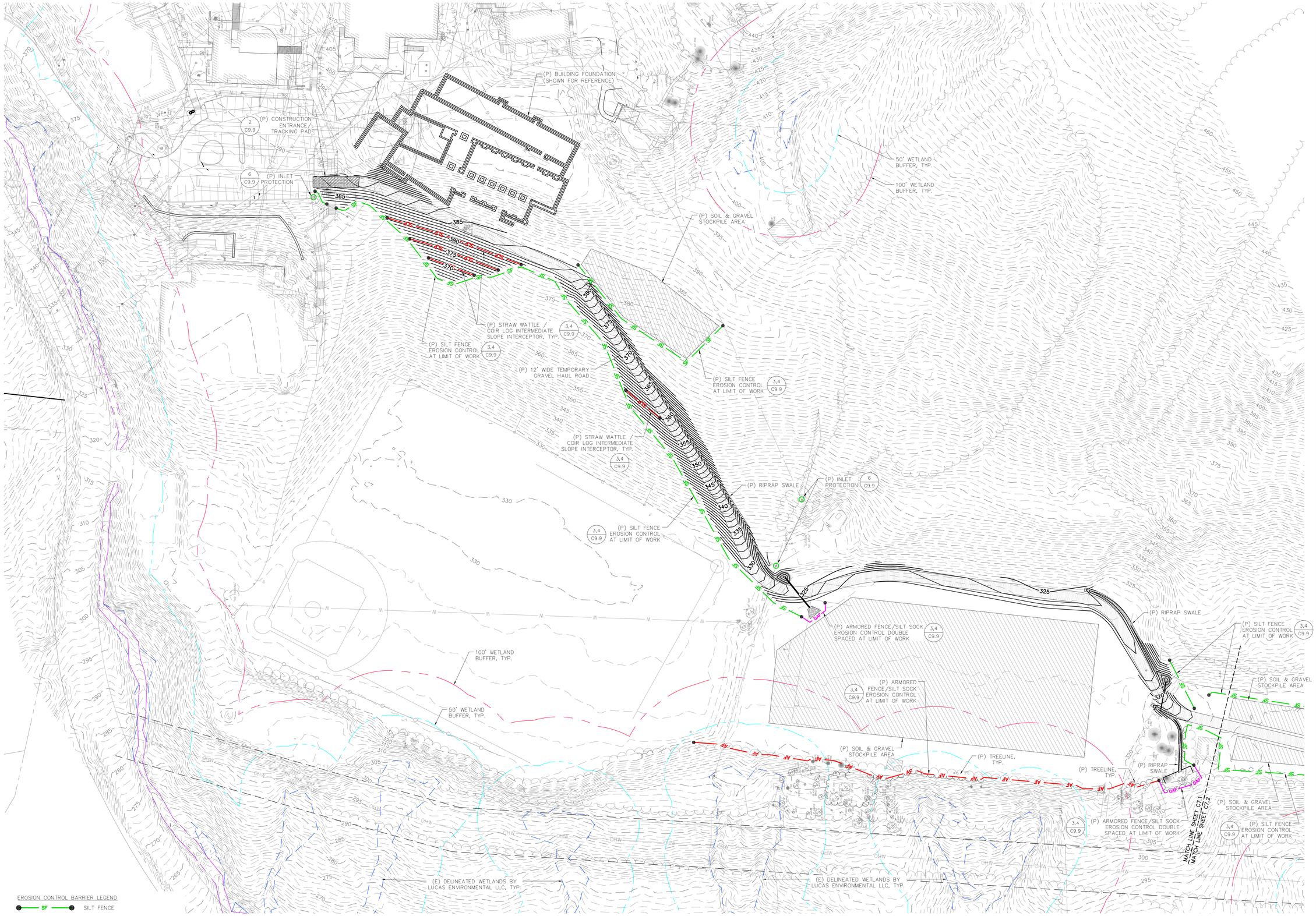
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
SWITCHGEAR RETAINING WALL PLAN & PROFILE

DRAWING NUMBER
C6.4

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EROSION CONTROL BARRIER LEGEND

- SF — SILT FENCE
- AF — ARMORED FENCE / SILT SOCK
- DAF — ARMORED FENCE / SILT SOCK DOUBLE SPACED
- SLP — STRAW WATTLE / COIR LOG INTERMEDIATE SLOPE INTERCEPTOR

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

TOTAL SITE DISTURBANCE:
TEMPORARY HAUL ROAD = 4.37± ACRES

EROSION CONTROL PLAN: TEMPORARY HAUL ROAD

1 / C7.1

SCALE: 1"=40' (30x42)

40 20 0 40 80

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WINDIGO
ARCHITECTURE, LLC

914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

WWW.WINDIGODESIGN.COM

SITE CONSULTANT

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DESIGN GROUP, LLC

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Suite 200
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	03/08/23	NOI SUBMITTAL

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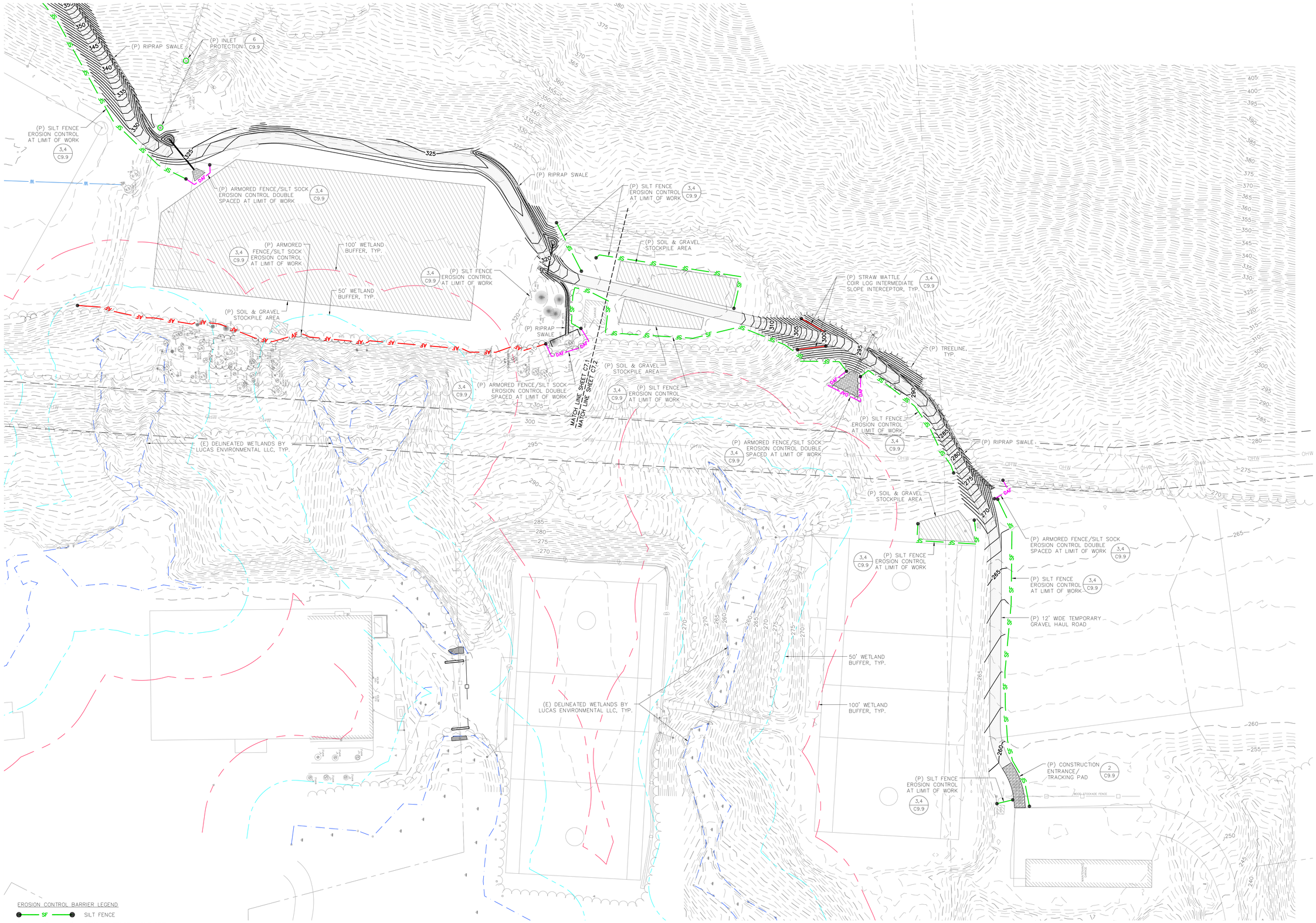
COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
No. 47815
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
EROSION CONTROL PLAN: TEMPORARY HAUL ROAD

DRAWING NUMBER
C7.1



EROSION CONTROL BARRIER LEGEND

- SF — SILT FENCE
- AF — ARMORED FENCE / SILT SOCK
- DAF — ARMORED FENCE / SILT SOCK DOUBLE SPACED
- SLP — STRAW WATTLE / COIR LOG INTERMEDIATE SLOPE INTERCEPTOR

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

EROSION CONTROL PLAN: TEMPORARY HAUL ROAD

1 / C7.2

SCALE: 1"=40' (30x42)

40 20 0 40 80

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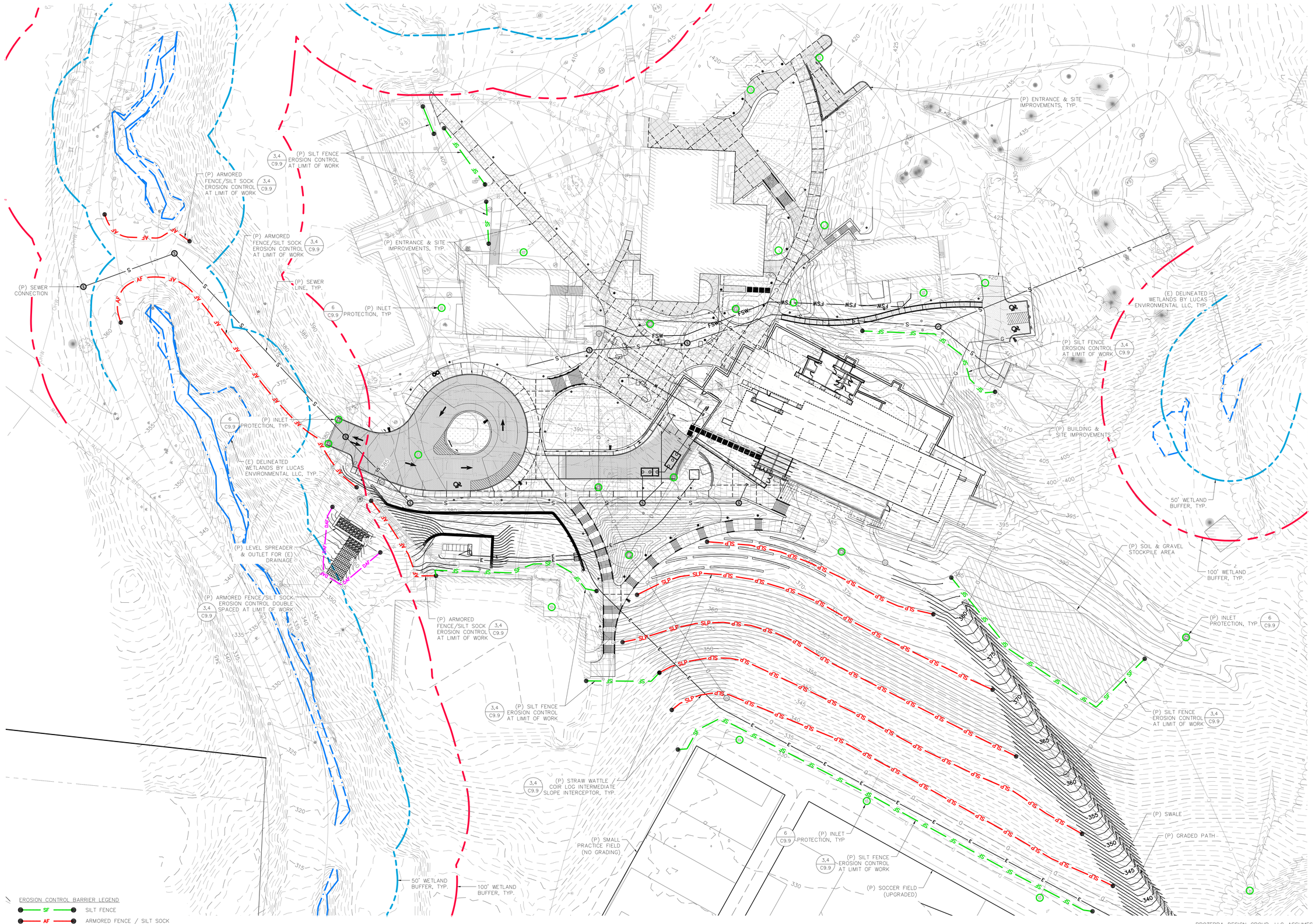
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
EROSION CONTROL PLAN: TEMPORARY HAUL ROAD

DRAWING NUMBER
C7.2

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EROSION CONTROL BARRIER LEGEND

SF	SILT FENCE
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DAF	ARMORED FENCE / SILT SOCK DOUBLE SPACED
SLP	STRAW WATTLE / COIR LOG INTERMEDIATE SLOPE INTERCEPTOR

DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

TOTAL SITE DISTURBANCE: FINAL GRADING = 9.4± ACRES

EROSION CONTROL PLAN: FINAL GRADING

SCALE: 1"=30' (30x42)

1 / C7.3

30 15 0 30 60

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	03/06/23	NOI SUBMITTAL

PROGRESS SET
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COMMONWEALTH OF MASSACHUSETTS
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CIVIL
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REGISTERED PROFESSIONAL ENGINEER
3-6-2023

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SCALE
SEE PLAN

DRAWING NAME
EROSION CONTROL PLAN: FINAL GRADING

DRAWING NUMBER
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(P) INLET PROTECTION, TYP. 6 C9.9

(P) SILT FENCE EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) DRAINAGE, TYP.

(P) ELECTRIC, TYP.

(P) SOCCER FIELD (UPGRADED)

(P) STRAW WATTLE / COIR LOG INTERMEDIATE SLOPE INTERCEPTOR, TYP. 3,4 C9.9

(P) INLET PROTECTION, TYP. 6 C9.9

(P) GRADED PATH

(P) SILT FENCE EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) SWALE

(P) INLET PROTECTION, TYP. 6 C9.9

(P) SILT FENCE EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) SOCCER FIELD (UPGRADED)

(P) FIELD UNDERDRAIN, TYP.

(P) UNDERGROUND DETENTION SYSTEM

(P) STRAW WATTLE / COIR LOG INTERMEDIATE SLOPE INTERCEPTOR, TYP. 3,4 C9.9

(P) SILT FENCE EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) SOIL & GRAVEL STOCKPILE AREA

100' WETLAND BUFFER, TYP.

(P) INLET PROTECTION, TYP. 6 C9.9

(P) RELOCATED TIMING SHED

(P) 550± LF RETAINING WALL 3,000± SF)

100' WETLAND BUFFER, TYP.

50' WETLAND BUFFER, TYP.

50' WETLAND BUFFER, TYP.

BEGIN WALL STA.=0+00

(P) ARMORED FENCE/SILT SOCK EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) GENERATOR AREA

(P) SOIL & GRAVEL STOCKPILE AREA

(P) ARMORED FENCE/SILT SOCK EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(P) ARMORED FENCE/SILT SOCK EROSION CONTROL DOUBLE SPACED AT LIMIT OF WORK 3,4 C9.9

(P) SILT FENCE EROSION CONTROL AT LIMIT OF WORK 3,4 C9.9

(E) DELINEATED WETLANDS BY LUCAS ENVIRONMENTAL LLC, TYP.

(P) ARMORED FENCE/SILT SOCK EROSION CONTROL DOUBLE SPACED AT LIMIT OF WORK 3,4 C9.9

(E) DELINEATED WETLANDS BY LUCAS ENVIRONMENTAL LLC, TYP.

(E) DELINEATED WETLANDS BY LUCAS ENVIRONMENTAL LLC, TYP.

EROSION CONTROL BARRIER LEGEND

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DOUBLE SPACED EROSION CONTROL BARRIERS SHALL BE LOCATED AT OUTFALLS AND CRITICAL POINTS.

EROSION CONTROL PLAN: FINAL GRADING
SCALE: 1"=30' (30x42)

1 C7.4

30 15 0 30 60

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	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

JESSE M. RIVIELLO
CIVIL
No. 47918
REGISTERED PROFESSIONAL ENGINEER
COMMONWEALTH OF MASSACHUSETTS
3-8-2023

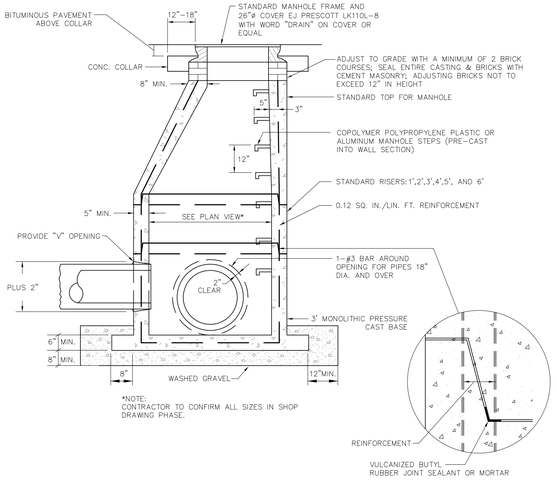
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
EROSION CONTROL PLAN: FINAL GRADING

DRAWING NUMBER
C7.4

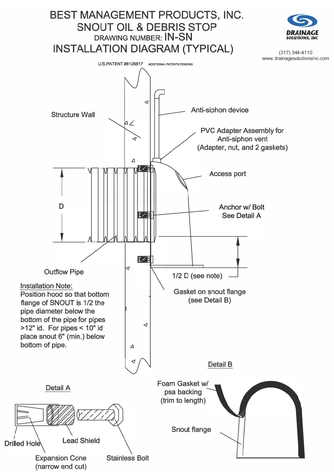
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STANDARD PRECAST DRAIN MANHOLE DETAIL

1 / C9.1

SCALE: NONE

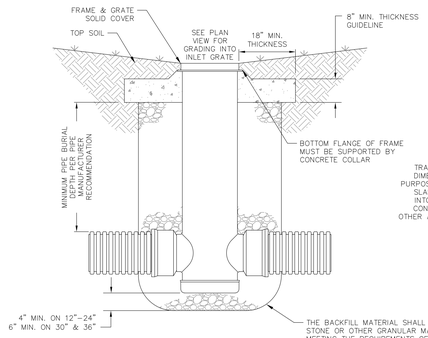


CATCH BASIN HOOD DETAIL

4 / C9.1

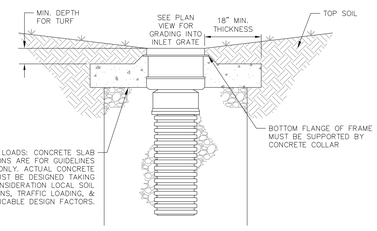
NOT TO SCALE

12" - 36" DRAIN BASIN WITH INTEGRATED HOOD



1. GRATES/SOLID COVERS SHALL MEET H–20 LOAD RATING FOR 30" PEDESTRIAN & 12" – 30" STANDARD & SOLID
2. DESIGN SHOULD ACCOUNT FOR ROOT DEPTH TO ALLOW TURF TO GROW AND PREVENT EROSION AROUND GRATE SO THAT HAZARDS DO NOT FORM.

12" - 30" INLINE DRAIN



Nyloplast

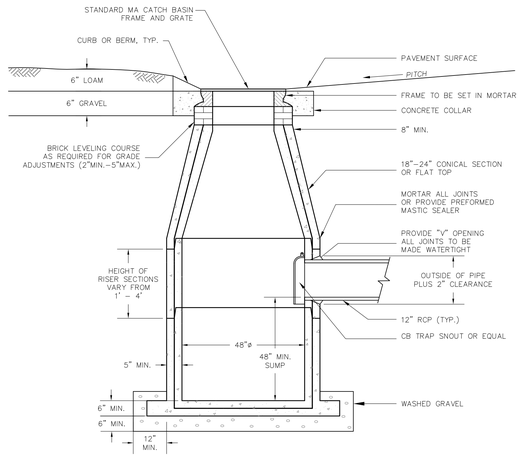
3130 VERONA AVE
BUFORD, GA 30518
PHN (770) 932-2443
FAX (770) 932-2490
www.nyloplast-us.com

DRAIN BASIN & INLINE DRAIN
TURF TRAFFIC INSTALLATION

NYLOPLAST TURF TRAFFIC INSTALLATION

2 / C9.1

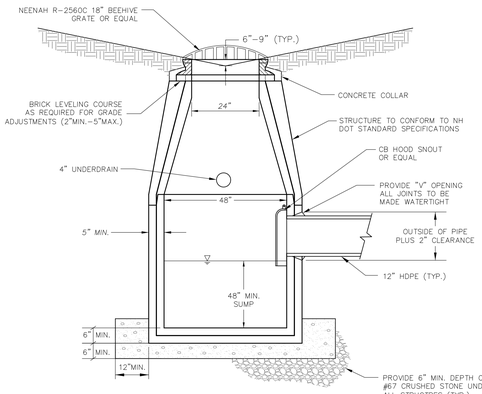
SCALE: NONE



PRECAST CONCRETE CATCH BASIN

3 / C9.1

SCALE: NONE



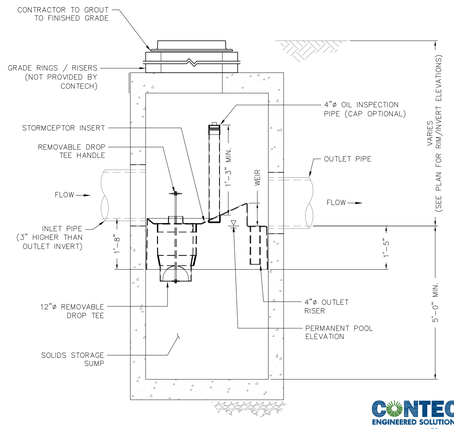
NOTE:

1. THIS STYLE CATCH BASIN SHALL BE USED WITHIN BIO–SWALES IN PARKING AREAS IN CONJUCTION WITH DRAIN–INLETS.

DROP-STYLE YARD BASIN

5 / C9.1

SCALE: NONE



SECTION A-A

CONTECH ENGINEERED SOLUTIONS LLC
www.contechES.com
9025 Centre Pointe Dr. Suite 400, West Chester, OH 45069
800-338-1122 513-645-7000 513-645-7993 FAX

GENERAL NOTES

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE, www.ContechES.com
3. STORMCEPTOR WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
4. STORMCEPTOR STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' – 2', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO.
5. STORMCEPTOR STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C478 AND AASHTO LOAD FACTOR DESIGN METHOD.

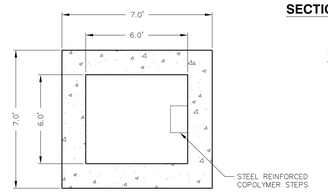
INSTALLATION NOTES

A. ANY SUB–BASE, BACKFILL DEPTH, AND/OR ANTI–FLOTATION PROVISIONS ARE SITE–SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE STORMCEPTOR MANHOLE STRUCTURE.
C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

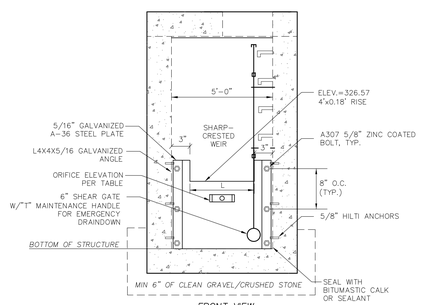
STORMCEPTOR STANDARD (STC450i) DETAIL

6 / C9.1

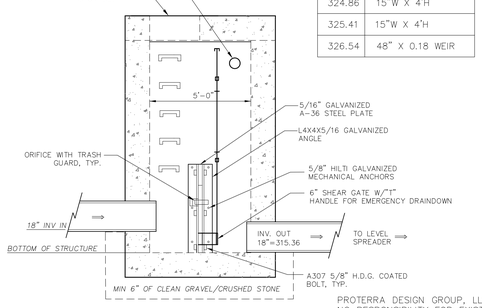
SCALE: NONE



PLAN



FRONT VIEW



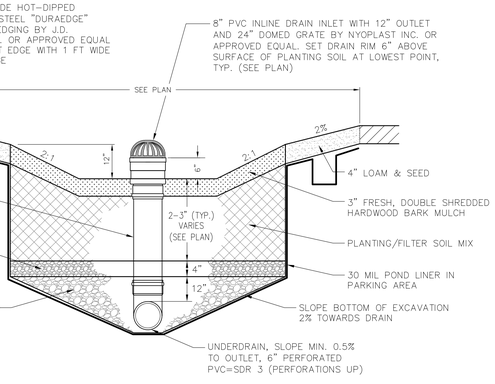
ORIFICE TABLE	
ELEV.	DESCRIPTION
323.51	4" VERT. ORIFICE
323.81	15"W X 4'H
324.86	15"W X 4'H
325.41	15"W X 4'H
326.54	48" X 0.18 WEIR

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TYPICAL UNDERGROUND BASIN OUTLET STRUCTURE

8 / C9.1

SCALE: NONE



RAINGARDEN TYPICAL CROSS-SECTION

7 / C9.1

SCALE: NONE

PARAMETER	VALUE
pH RANGE	5.5 TO 6.5
ORGANIC MATTER	1.5 TO 3.0% (BY WEIGHT)
MAGNESIUM	35 LBS. PER ACRE, MINIMUM
PHOSPHORUS (PHOSPHATE P_2O_5)	75 LBS. PER ACRE, MINIMUM
POTASSIUM (POTASH–K_2O)	85 LBS. PER ACRE, MINIMUM
SOLUBLE SLATS	≤ 500 PPM
TOP SOIL	20 TO 30%
COMPOST	30 TO 40%
SAND	40 TO 50%

FILTER MEDIA NOTES:

A PERMEABILITY OF AT LEAST 1.0 FEET PER DAY (0.5"/HR) IS REQUIRED.

SAND COMPONENT SHALL BE GRAVELLY SAND THAT MEETS ASTM D 422.

SOIL MIX MUST BE UNIFORM, FREE OF STONES, STUMPS, ROOTS, ROCKS, OR SIMILAR OBJECTS LARGER THAN 2".

CLAY CONTENT SHALL NOT EXCEED 5%.

TOP SOIL SHALL BE A SANDY LOAM, LOAMY SAND, OR LOAM TEXTURE.

COMPOST COMPONENT MUST BE PROCESSED FROM YARD WASTE IN ACCORDANCE WITH MASS DEP GUIDELINES. THE COMPOST SHALL NOT CONTAIN BIOSOLIDS.

PLANTING/FILTER SOIL CHARACTERISTICS

WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
Ph:(413)320–4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47915
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

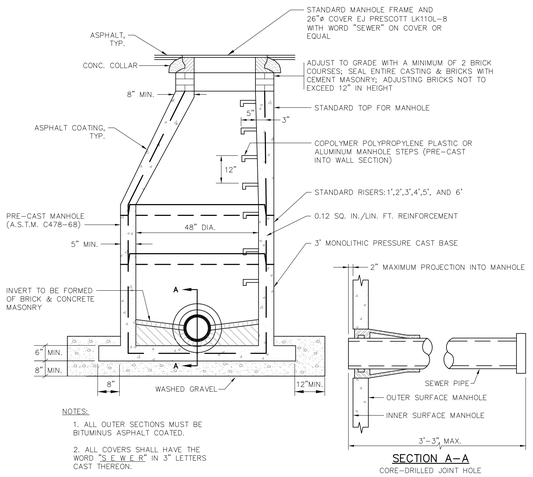
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.1

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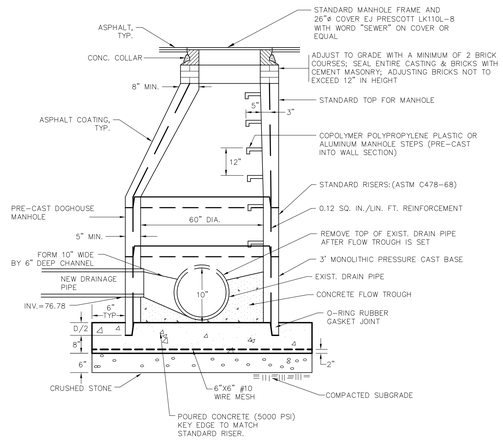
NOTES:

1. ALL OUTER SECTIONS MUST BE BITUMINUS ASPHALT COATED.
2. ALL COVERS SHALL HAVE THE WORD "S E W E R" IN 3" LETTERS CAST THEREON.

STANDARD PRECAST SEWER MANHOLE DETAIL

1 / C9.2

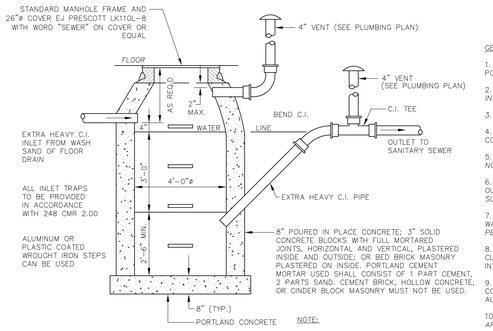
SCALE: NONE



"DOGHOUSE" SEWER MANHOLE W/CAST IN PLACE BASE

2 / C9.2

SCALE: NONE



NOTE:

PRE-CAST SEPARATORS ARE TO HAVE ALL SPECIFIED HOLES EITHER CORE-BORED OR CAST IN PLACE.

CIRCULAR BASINS ARE RECOMMENDED.

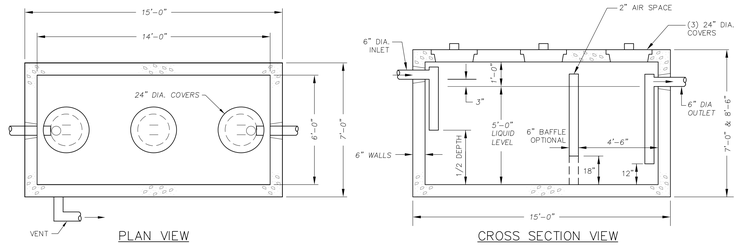
GENERAL CONSTRUCTION NOTES:

1. BASIN TO BE LOCATED OUTSIDE OF BUILDING WHERE POSSIBLE, COVER TO HAVE A CENTER HOLE.
2. A TIGHT COVER MUST BE USED IF BASIN IS LOCATED *INSIDE OF BUILDING*
3. OPENING SHALL BE NOT LESS THAN 24" DIA..
4. THE CATCH BASIN SHALL BE SO LOCATED AND CONSTRUCTED THAT SURFACE WATER SHALL BE EXCLUDED.
5. INLET PIPE SHALL BE AT LEAST FOUR INCHES ABOVE *NORMAL WATER LINE.*
6. WHERE SUBJECT TO FROST OR CRUSHING CONDITIONS, OUTLET SHALL BE AT LEAST THREE FEET BELOW THE *SURFACE.*
7. THE NEW CATCH BASIN MUST BE FILLED WITH CLEAN WATER BEFORE USING, AND AFTER BEING EMPTIED FOR *PERIODIC CLEANING.*
8. ALL OIL AND GASOLINE MUST BE REMOVED BEFORE CLEAING OUT THE BASIN, AND MUST NOT BE DISCHARGED INTO THE SEWER THROUGH OTHER FIXTURES.
9. SPECIFICATIONS FOR COVERING SPECIAL CASES OR CONDITIONS, SHALL BE APPROVED BY THE LOCAL AUTHORITIES OF THE M.W.R.A..
10. WROUGHT IRON STEPS SHALL BE SPACED ABOUT 18" *APART.*
11. BOTH VENTS SHALL BE EXTENDED INDEPENDENTLY 18" ABOVE THE ROOF, OR AS APPROVED BY THE LOCAL AUTHORITIES, HAVING JURISDICTION.

OIL AND GREASE SEPARATOR

3 / C9.2

SCALE: NONE



NOTES:

1. PROPERLY SIZED GREASE TRAP SHALL BE INSTALLED TO INTERCEPT FATS & OILS FROM KITCHEN FLOW OF RESTAURANTS & COMMERCIAL KITCHENS.
2. 24" PAMREX HINGED DUCTILE IRON COVERS WITH GASKETS SHALL BE BROUGHT TO GRADE OVER EACH OPENING.
3. GREASE TRAPS SHALL BE VENTED THROUGH BUILDING PLUMBING AS REQUIRED BY CODE.
4. 5' MAXIMUM EARTH COVER.

GREASE TRAP SIZING FOR COMMERCIAL KITCHENS BASED UPON SEATING

# SEATS	TITLE 5 STATE ENVIRONMENTAL CODE (GAL)	MASS. PLUMBING CODE (GAL)	PROPOSED SIZING (GAL)	LENGTH	WIDTH	HEIGHT	MODEL NUMBER	RATING
300	4500	6000	3000 (2 EA.)	14'-0"	6'-0"	6'-0"	GT3000	HS-20
350	**5250**	**7000**	**4000 (2 EA.)**	**14'-0"**	**6'-0"**	**7'-6"**	**GT4000**	**HS-20**

TITLE 5 (MIN. CAPACITY PF 1,000 GALLONS OR 24 HOUR DETERMINED FOR KITCHEN FLOW) 15.230
15 GPD/SEAT (KITCHEN FLOW FROM 310 CMR 15.203) SECONDARY (MIDDLE SCHOOL W/CAFETERIA (NO GYM OR SHOWERS)
SEATS = 350 350 X 15gpd/SEAT = 5250 GALLONS

PLUMBING CODE (248 CMR 10.09 TABLE 2) OTHER ESTABLISHMENTS WITH COMMERCIAL KITCHENS.
P X M X GM X LF = EFFECTIVE CAPACITY OF GREASE TRAPS AND INTERCEPTORS IN GALLONS
312.5 X 4 X 5 X 1 = 6250 GALLONS

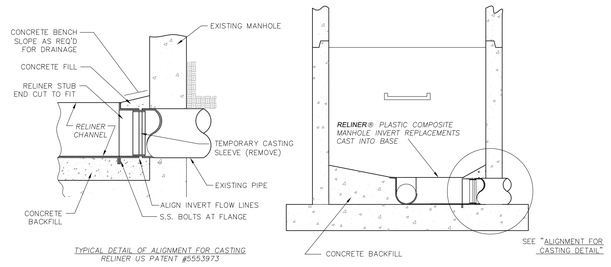
WHERE:
M = MEALS PREPARED PER DAY = 4 (B) BREAKFAST, (L) LUNCH, (D) DINER, (S) SNACK
GM = GALLONS OF WASTE WATER PER MEAL (USE 5 GALLONS)
P = NUMBER OF RESIDENTS AND/OR PATRONS
= 300 FOR BREAKFAST, DINNER, AND SNACK; 350 FOR LUNCH
= 312.5 AVG.
LF = LOADING FACTOR (USE 1.0 WITH DISHWASHING MACHINE)

NOTE: BASED UPON THE ASSUMPTIONS PROVIDED WE HAVE ESTIMATED THE GREASE TRAP SIZES LISTED ABOVE. MANY FACTORS WHICH WILL EFFECT ACTUAL SIZING OF GREASE TRAPS WILL BE DEPENDENT ON BOTH THE NUMBER OF SEATS, HOURS OF OPERATION, AND TYPE OF ESTABLISHMENT WHICH ARE NOT FULLY KNOWN AT THIS TIME. THE CONTRACTOR SHALL CONFIRM WITH THE ENGINEER SIZING OF THE GREASE TRAPS USING THE LATEST INFORMATION ON THE PROPOSED DINING HALL.

DINING HALL 5,000 GALLON GREASE TRAP

4 / C9.2

SCALE: NONE



TYPICAL DETAIL OF ALIGNMENT FOR CASTING
RELINER US PATENT #5553973

NOTES:

1) ALL MANHOLE STRUCTURES SHALL BE OUTFITTED WITH RELINER®PLASTIC COMPOSITE INVERT COMPONENTS PRODUCED BY:

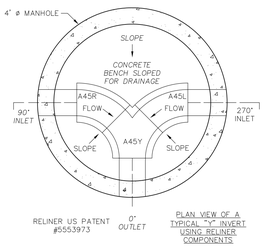
RELINER/DURAN, INC.
53 MT. ARCHER ROAD
LYME, CT. 06371
www.reliner.com
(800) 508-6001 FAX: (860) 434-3195

2) ALL CHANNELS SHALL BE AT LEAST THE SAME DEPTH AS THE ABOVE PIPE AND SHALL BE FIELD TRIMMED AND ASSEMBLED AS PER MANUFACTURERS INSTRUCTIONS.

3) CONCRETE BACK FILL SHALL BE PROPERLY PLACED AND GRADED SO AS TO PROVIDE SMOOTH, WELL-DRAINED SLOPING BENCHES.

4) PIPE ALIGNMENT SHALL BE MAINTAINED DURING BACK FILL BY USE OF EXTERNAL ADAPTERS , INFLATABLE PLUGS OR TEMPORARY INTERNAL CASTING SLEEVES (SEE DETAIL).

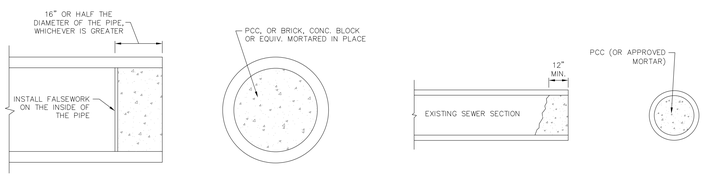
5) CONTRACTOR SHALL INSPECT EACH MANHOLE AND VERIFY COMPONENTS REQUIRED TO OUTFIT EACH STRUCTURE.



SEWER INVERT DETAIL

5 / C9.2

SCALE: NONE



FOR PLUGGING EXISTING SEWER (OVER 18" DIA.)

STANDARD SEWER PLUG

7 / C9.2

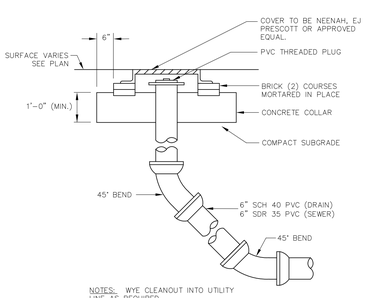
SCALE: NONE

FOR PLUGGING EXISTING SEWER (18" AND SMALLER)

STANDARD SEWER PLUG

8 / C9.2

SCALE: NONE

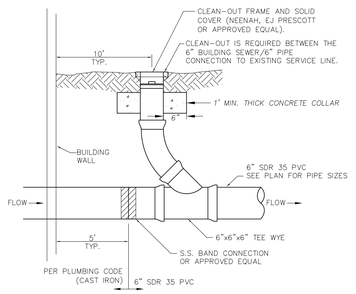


NOTES: WYE CLEANOUT INTO UTILITY LINE AS REQUIRED

TYPICAL STORM OR SEWER CLEAN-OUT DETAIL

9 / C9.2

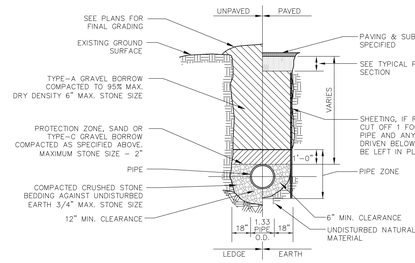
SCALE: NONE



TYPICAL BUILDING SEWER CONNECTION

10 / C9.2

SCALE: NONE



NOTE:

1. GRAVEL BORROW SHALL CONFORM TO MASSDOT SPECIFICATION M1.03.0
2. CRUSHED STONE BEDDING SHALL CONFORM TO MASSDOT SPECIFICATION M2.01.1
3. CONTRACTOR TO NOTIFY ENGINEER IMMEDIATELY UPON DISCOVERY OF UNSUITABLE FOUNDATION MATERIALS OR HIGH GROUNDWATER WHICH COMPROMISE UTILITY INSTALLATION. UNFORESEEN CONDITIONS MAY REQUIRE AN ALTERNATIVE TRENCH SECTION WITH ADDITIONAL STONE AND FILTER FABRIC.

GRAVITY SEWER SERVICE TRENCH

6 / C9.2

SCALE: NONE

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WINDIGO
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914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

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DESIGN GROUP, LLC
4 Bay Road, Bldg A
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	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

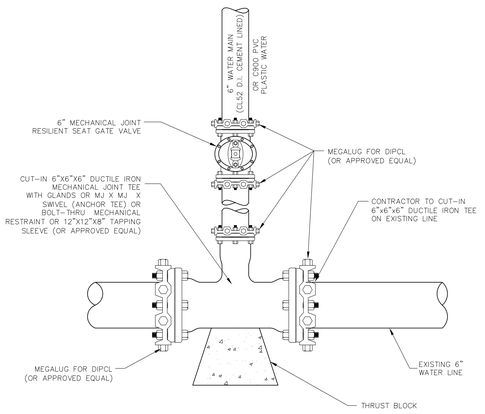
COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47916
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.2



NOTES:
1. ALL MATERIALS AND INSTALLATION PROCEDURES SHALL CONFORM TO TOWN OF DEERFIELD WATER DEPARTMENT STANDARDS.
2. ALL PIPE SHOULD HAVE A MINIMUM DEPTH OF 5' FROM TOP OF PIPE TO FINISH GRADE.
3. THE MECHANICAL JOINTS OF THE PIPES BETWEEN THE VALVES AND THE FITTINGS SHALL BE RESTRAINED VIA MEGA-A-LUG JOINT CLAMPS (OR APPROVED EQUAL).

STANDARD CUT-IN TEE INSTALLATION
1 / C9.3
SCALE: NONE

TYPE A BLOCKING
FOR 11 1/4° & 22 1/2° VERT BENDS

PIPE SIZE (NOM DIAMETER) IN INCHES	VERTICAL BEND DEGREES	NO. OF CUFT OF CONC BLOCKING	SIDE OF CUBE (FEET)	DIA OF SHACKLE RODS(2) * (INCHES)	DEPTH OF RODS IN CONC (FEET)
4"	11 1/4°	8	2.0	3/4"	1.6
	22 1/2°	16	2.5		
6"	11 1/4°	16	2.5	3/4"	1.6
	22 1/2°	32	3.2		
8"	11 1/4°	28	3.0	3/4"	1.6
	22 1/2°	55	3.8		
10"	11 1/4°	42	3.5	3/4"	1.6
	22 1/2°	83	4.4		
12"	11 1/4°	60	3.9	3/4"	1.6
	22 1/2°	118	4.9	7/8"	2.2
16"	11 1/4°	104	4.7	7/8"	2.2
	22 1/2°	205	5.9	1 1/8"	3.7

TYPE B BLOCKING
FOR 45° VERTICAL BENDS

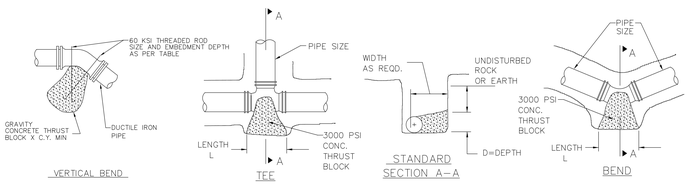
PIPE SIZE, NOM DIAMETER IN INCHES	NO. OF CUFT OF CONC BLOCKING	SIDE OF CUBE (FEET)	DIA OF SHACKLE RODS(2) * (INCHES)	DEPTH OF RODS IN CONC (FEET)
4"	29	3.1	3/4"	1.6
6"	59	3.9		
8"	102	4.7		
10"	154	5.4		
12"	218	6.0		
16"	378	7.2	1 1/8"	3.7

THRUST BLOCK NOTES

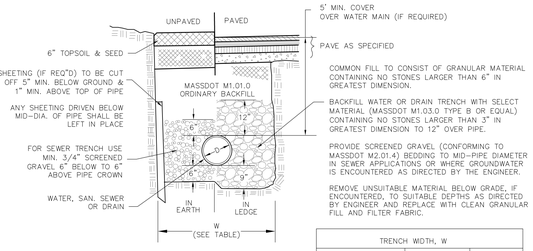
1. FOR REQUIRED BEARING AREA DIMENSIONS D & L SEE TABLE. DIMENSIONS OF D & L OTHER THAN THOSE SHOWN IN THE TABLE MAY BE USED PROVIDED THEY YIELD A BEARING AREA EQUAL TO OR LARGER THAN THAT REQUIRED.
2. CONCRETE NOT TO OVERLAP ANY JOINT.
3. CONCRETE TO BE PLACED SO AS NOT TO INTERFERE WITH REMOVING OR INSTALLING ANY OF THE JOINTING HARDWARE.
4. APPROXIMATE VOLUME OF CONCRETE THRUST BLOCK:
   $V = \frac{LD\,(W+ID) - ID}{81}$
   WHERE:
   V = VOLUME IN CUBIC YARDS
   L = LENGTH OF BLOCK IN FEET
   D = DEPTH OF BLOCK IN FEET
   W = WIDTH OF BLOCK IN FEET
   ID = INSIDE DIAMETER OF PIPE IN FEET
5. VALUES FOR TEE ALSO APPLY TO END PLUGS, CAPS, AND TAPPING SLEEVES.
6. REQUIRED BEARING AREAS ARE DUE TO THRUSTS CAUSED BY 150 PSI WORKING PRESSURE PLUS 50%(75 PSI) SURGE ALLOWANCE RESULTING IN 225 PSI TOTAL INTERNAL PRESSURE. NORMAL PIPE DIAMETER USED.
7. REQUIRED BEARING AREAS ARE BASED ON ALLOWABLE SOIL BEARING CAPACITY OF 2000 LBS. PER SQUARE FOOT FOR SAND. DUE TO OTHER SOIL CONDITIONS ENCOUNTERED, BEARING AREAS MAY BE MODIFIED BY THE ENGINEER.
8. IN MUCK, PEAT, OR RECENTLY PLACED FILL ALL THRUST SHALL BE RESISTED BY PILES OR TIE RODS TO SOLID FOUNDATIONS, OR BY REMOVAL OF SUCH UNSTABLE MATERIAL AND REPLACEMENT WITH BALLAST OF SUFFICIENT STABILITY TO RESIST THE THRUSTS, ALL AS REQUIRED BY THE ENGINEER.

REQUIRED BEARING AREAS & DIMENSIONS
FOR CONCRETE THRUST BLOCKS

PIPE SIZE (IN.)	TEE(See Note 5) AREA Sq.Ft.	TEE(See Note 5) Dimen. D x L	90°(1/4)BEND AREA Sq.Ft.	90°(1/4)BEND Dimen. D x L	45°(1/8)BEND AREA Sq.Ft.	45°(1/8)BEND Dimen. D x L	22-1/2°(1/16)BEND AREA Sq.Ft.	22-1/2°(1/16)BEND Dimen. D x L	11-1/4°(1/32)BEND AREA Sq.Ft.	11-1/4°(1/32)BEND Dimen. D x L
3 & 4	1.4	1.0 x 1.5	2.0	1.0 x 2.0	1.1	1.0 x 1.5	0.6	0.5 x 1.5	0.3	0.5 x 1.0
6	3.2	1.5 x 2.5	4.5	2.0 x 2.5	2.4	1.5 x 2.0	1.2	1.0 x 1.5	0.6	1.5 x 1.5
8	5.7	2.0 x 3.0	8.0	2.0 x 4.0	4.3	2.0 x 2.5	2.2	1.5 x 1.5	1.1	1.0 x 1.5
12	12.7	3.5 x 3.5	18.0	4.0 x 4.5	9.7	2.5 x 4.0	5.0	2.0 x 2.5	2.5	1.5 x 2.0
16	50.0	6.0 x 8.5	50.0	6.0 x 8.5	27.0	5.0 x 5.5	13.8	3.5 x 4.0	6.9	2.5 x 3.0



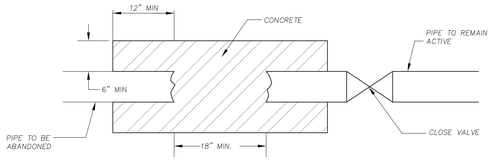
THRUST BLOCK DETAILS
2 / C9.3
SCALE: NONE



TRENCH WIDTH, W		
D DIAMETER OF PIPE	W UNSHEETED	W SHEETED
TO 12"	3'	4'
14" TO 24"	4'	5'
30" TO 36"	5'	6'

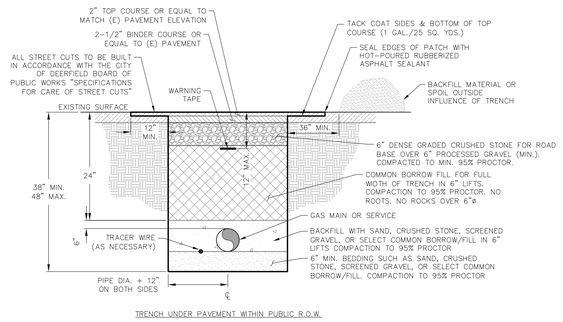
NOTES:
1. ALL TRENCH CONSTRUCTION TO CONFORM TO APPLICABLE FEDERAL, STATE AND LOCAL REGULATIONS.
2. COMPACT FILL AND TAMP PIPE TO 95% MAX. DRY DENSITY IN 8" LIFTS UNLESS OTHERWISE SPECIFIED.
3. INSTALL DUCTILE IRON WATER PIPE IN ACCORDANCE WITH ANSI A21.51 (AWWA C151) LAYING CONDITION TYPE 2. BACKFILL TO CONFORM TO MASSDOT M1.03.0 GRAVEL BURROW TYPE "C" TO 12" ABOVE PIPE CROWN OR AS DIRECTED BY MANUFACTURER OR ENGINEER.
4. MATERIALS FOR SEWER BEDDING, HAUNCHING, AND BACKFILL TO CONFORM TO CLASSES I, II, OR III AS DESCRIBED IN ASTM D 2321 AND TR-16 GUIDES FOR THE DESIGN OF WASTEWATER TREATMENT WORKS.
5. BC TO 12" ABOVE PIPE CROWN OR AS DIRECTED BY MANUFACTURER OR ENGINEER.
6. PROVIDE MINIMUM 5 FT. COVER OVER WATER MAIN AS MEASURED FROM BOTTOM OF CURB LINE. INSULATE WATERMAIN IN ACCORDANCE WITH MASSDOT SECTION 301 WATER SYSTEMS IN AREAS PRONE TO FROST ACTION AND/OR LESS THAN 5' MIN. COVER OR AS DEERFIELD WATER DEPARTMENT REQUIRES.

TYPICAL TRENCH SECTION
4 / C9.3
SCALE: NONE



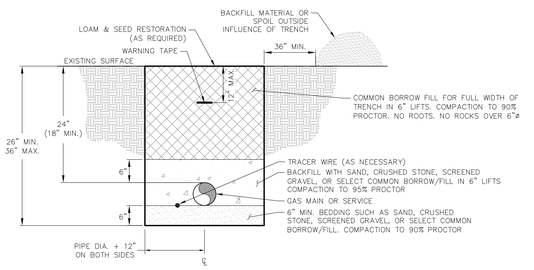
NOTE: PLACE CONCRETE A MINIMUM OF 12 INCHES INSIDE DISCONTINUED PIPE.

PLUGGED AND ABANDONED PIPE DETAIL
7 / C9.3
SCALE: NONE



TRENCH UNDER PAVEMENT WITHIN PUBLIC R.O.W.

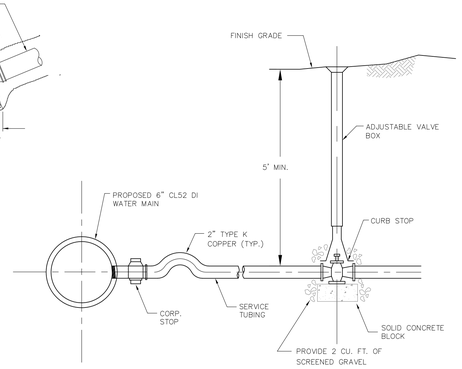
NOTES:
1. ALL MATERIALS & INSTALLATION PROCEDURES TO CONFORM TO THE CITY OF DEERFIELD DPW STANDARDS.
2. REQUIREMENTS FOR SUBBASE AND BASE MATERIAL TYPE ARE TO BE IN ACCORDANCE WITH LOCAL AUTHORITY HAVING JURISDICTION IN PAVED AREAS.
3. MILL TO REMOVE TOP COURSE.
4. SAW CUT EDGE OF UTILITY PATCH IF NO MILLING IS REQUIRED.



TRENCH UNDER SHOULDER OR UNPAVED

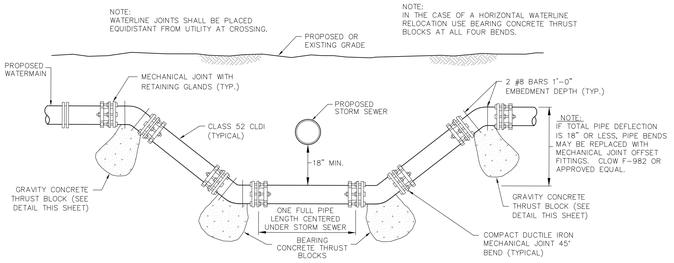
NOTES:
1. ALL GAS MATERIALS & INSTALLATION PROCEDURES TO CONFORM TO EVERSOURCE GAS OF MASSACHUSETTS AND MASSACHUSETTS GAS CODES.
2. REQUIREMENTS FOR GRAVEL, LOAM AND/OR SEED ARE TO BE IN ACCORDANCE WITH LOCAL AUTHORITY HAVING JURISDICTION IN NON-PAVED AREAS.
3. ALL GRAVEL MATERIALS USED TO MEET STATE DOT STANDARD SPECIFICATIONS FOR HIGHWAYS & BRIDGES AND THE CITY OF DEERFIELD STANDARDS.
4. AFTER TRENCH WORK IS COMPLETE, FILL AROUND PIPE TO BOTTOM WITH GRADED GRAVEL FILL AND COMPACT IN 6" LIFTS.
5. EXCAVATION FOR NATURAL GAS TO CONFORM TO EVERSOURCE GAS OF MASSACHUSETTS OPERATING AND MAINTENANCE PROCEDURES.
6. COMPACT TRENCH BACKFILL TO SPRING LINE/CROWN OF PIPE TO 95% MAX DRY DENSITY IN COMPLIANCE WITH AASHTO T-180. LIFTS SHALL NOT EXCEED 8".
7. IF COVER IS LESS THAN 2' OR ATTACHED TO STRUCTURES, SPECIAL DESIGN MAY BE REQUIRED.
8. NATIVE BACKFILL SHALL BE OBTAINED FROM EXCAVATION AND SHALL BE FREE FROM STONES, LUMPS, BROKEN CONCRETE OR BIT. CONC. EXCEEDING 4" IN DIMENSION AND DELETERIOUS MATERIAL.
9. IF MATERIAL IS DEEMED UNSUITABLE AS BACKFILL BY ENGINEER, FILL SHALL BE DISPOSED OF AND SUITABLE MATERIAL FURNISHED.

NATURAL GAS SERVICE
9 / C9.3
SCALE: NONE

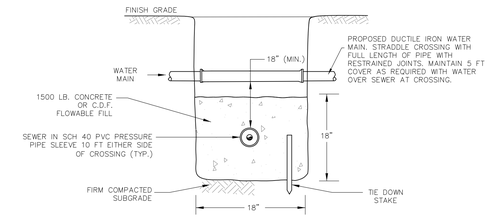


NOTE:
1. CONTRACTOR TO PROVIDE CATALOG CUTS OF ALL BRASS AND WATER VALVE COMPONENTS IN ACCORDANCE WITH LOCAL PREFERENCES.

SERVICE CONNECTION DETAIL
5 / C9.3
SCALE: NONE



WATERLINE OBSTRUCTION DETAIL
3 / C9.3
SCALE: NONE



NOTE:
1. CONTRACTOR TO PROVIDE SCH40 PIPE SLEEVE AND CONCRETE ENCASEMENT FOR EFFLUENT SEWER WHEN 10 FT HORIZONTAL SEPARATION CANNOT BE MAINTAINED OR AT UTILITY CROSSING.

WATER MAIN/SEWER CROSSING SLEEVE
6 / C9.3
SCALE: NONE

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WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT
ProTerra DESIGN GROUP, LLC
4 Bay Road, Bldg A Suite 200
Hadley, MA 01035
Ph:(413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
	06/30/22	DD PROGRESS SET
	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET NOT FOR CONSTRUCTION

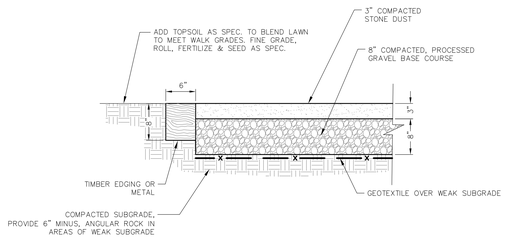


PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

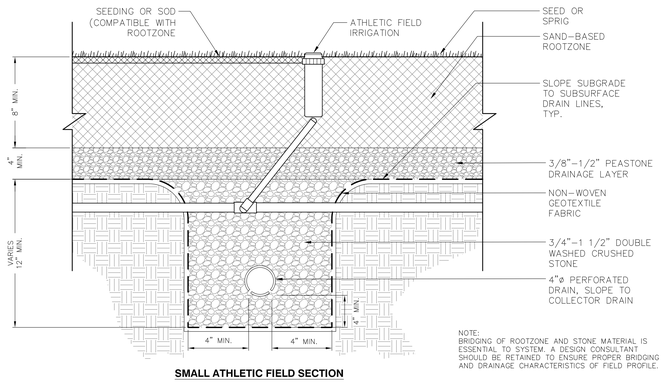
DRAWING NUMBER
C9.3



STONE DUST WALK

SCALE: NONE

1 / C9.4



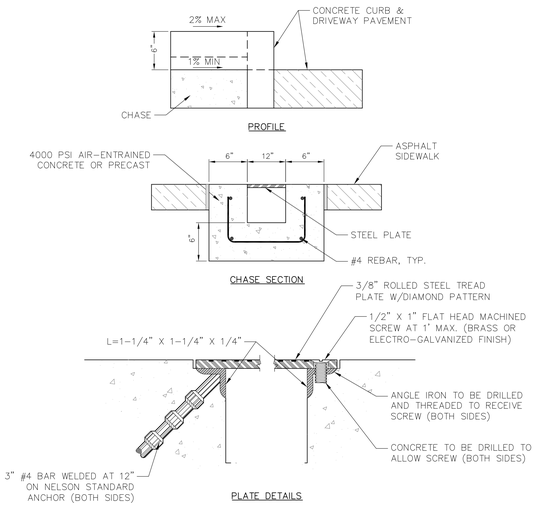
SMALL ATHLETIC FIELD SECTION

NOTE:
BRIDGING OF ROOTZONE AND STONE MATERIAL IS ESSENTIAL TO SYSTEM. A DESIGN CONSULTANT SHOULD BE RETAINED TO ENSURE PROPER BRIDGING AND DRAINAGE CHARACTERISTICS OF FIELD PROFILE.

SAND-BASED ROOTZONE ATHLETIC FIELD

SCALE: NONE

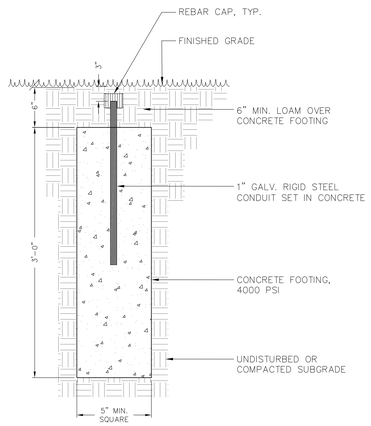
2 / C9.4



SIDEWALK PASS-THRU DRAIN

SCALE: NONE

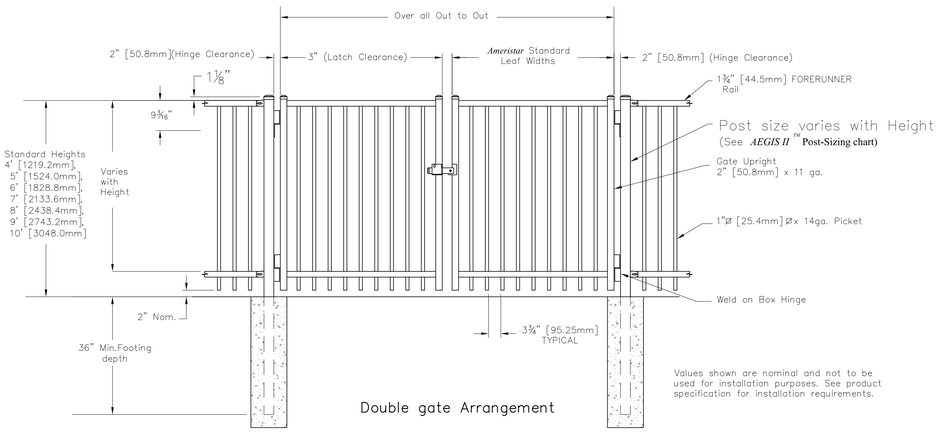
3 / C9.4



FIELD CORNER MONUMENT

SCALE: NONE

4 / C9.4

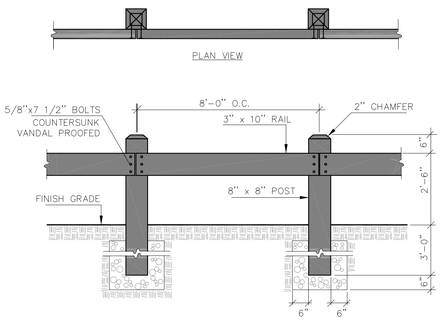


Values shown are nominal and not to be used for installation purposes. See product specification for installation requirements.

METAL FENCE

SCALE: NONE

5 / C9.4

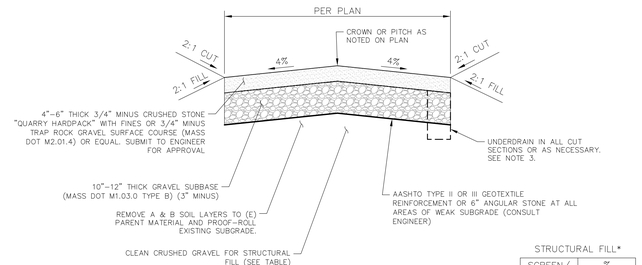


NOTES:
1. ALL WOOD TO BE PRESSURE TREATED, SELECT PINE GRADE 'B' OR BETTER
2. ALL HARDWARE TO BE GALVANIZED
3. SET POSTS 3' – 6" DEEP IN WELL-TAMPED GRAVEL BACKFILL

WOODEN GUARDRAIL

SCALE: NONE

10 / C9.4



NOTES:
1. ALL GRANULAR MATERIALS SHALL BE COMPACTED TO 95±% MAXIMUM DRY DENSITY BY STD. PROCTOR METHOD
2. ALL CUT AND FILL SLOPES TO BE COVERED WITH JUTE MESH EROSION CONTROL BLANKETS AND/OR HYDROSEEDED WITH NEW ENGLAND ROADSIDE MATRIX MIX AT 35LBS PER ACRE OR EQUAL.
3. THE REQUIREMENT FOR INSTALLATION OF SUBDRAINS SHALL BE AT ALL CUT SECTIONS OR AS DETERMINED BY THE ENGINEER-OF-RECORD BASED ON FIELD CONDITIONS AT THE TIME OF CONSTRUCTION. SUBDRAINS SHALL BE OUTLETTED TO DAYLIGHT AS APPROVED BY THE ENGINEER-OF-RECORD.
4. ALL MATERIALS TO CONFORM TO STATE AND/OR MUNICIPAL STANDARD AS REQUIRED.

STRUCTURAL FILL*

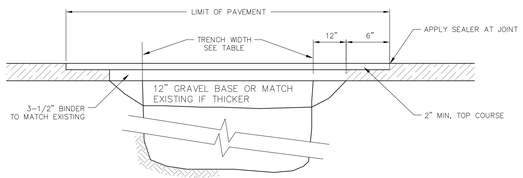
SCREEN/ SIEVE SIZE	% PASSING
3"	100
1/2"	50-85
#4	40-70
#50	8-28
#200	0-10

*ALTERNATES: MassDOT M1.03., TYPE A

DRIVEWAY HAUL ROAD SECTION

SCALE: NONE

8 / C9.4

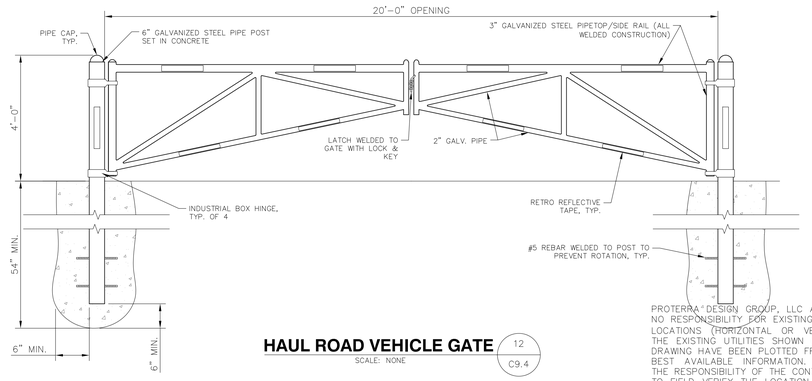


TRENCH WIDTH, W		
D DIAMETER OF PIPE	W UNSHEETED	W SHEETED
TO 12"	3'	4'
14" TO 24"	4'	5'
30" TO 36"	5'	6'

PAVEMENT PATCH DETAIL FOR TRENCH SECTIONS

SCALE: NONE

9 / C9.4



HAUL ROAD VEHICLE GATE

SCALE: NONE

12 / C9.4

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WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
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Proposed Dining Hall
FOR
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LOCATED AT
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ISSUE

NO.	DATE	DESCRIPTION
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	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. WINDIGO
CIVIL
No. 47219
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

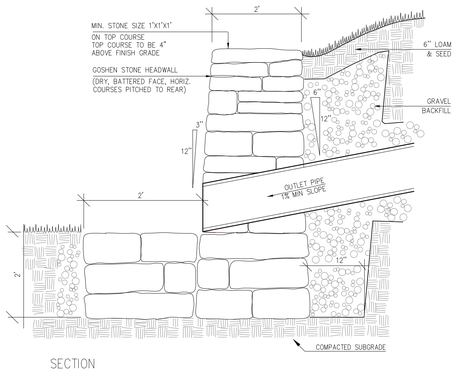
PROJECT NUMBER
22008

SCALE
SEE PLAN

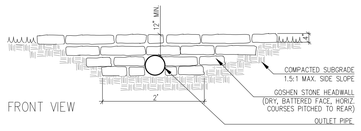
DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.4

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SECTION



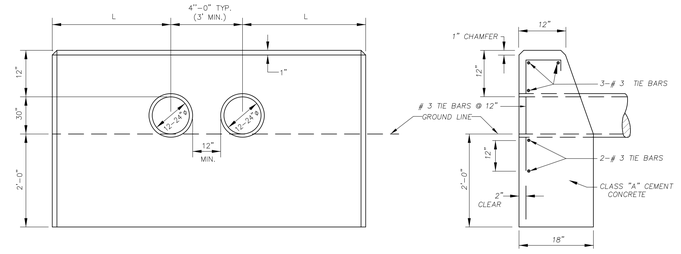
FRONT VIEW

GOSHEN STONE HEADWALL

NOT TO SCALE

1 C9.5

PIPE DIAM.	1.5 : 1 SLOPE	2 : 1 SLOPE
D	L	L
8"	4'-2"	5'-10"
10"	4'-10"	6'-8"
12"	5'-6"	7'-6"
15"	6'-6"	8'-9"
18"	7'-6"	10'-0"
21"	8'-6"	11'-6"
24"	9'-3"	12'-6"
30"	10'-6"	15'-0"
Q	4" FOR 1.5 : 1 SLOPE	
	6" FOR 2 : 1 SLOPE	



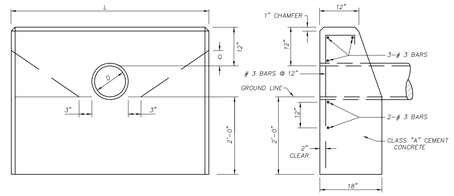
FRONT ELEVATION

END ELEVATION

CONCRETE HEADWALL FOR DUAL PIPE

NOT TO SCALE

2 C9.5



FRONT ELEVATION

END ELEVATION

PIPE DIAM.	1.5 : 1 SLOPE	2 : 1 SLOPE
D	L	L
12"	5'-6"	7'-6"
15"	6'-6"	8'-9"
18"	7'-6"	10'-0"
21"	8'-6"	11'-6"
24"	9'-3"	12'-6"
30"	10'-6"	15'-0"
Q	4" FOR 1.5 : 1 SLOPE	
	6" FOR 2 : 1 SLOPE	

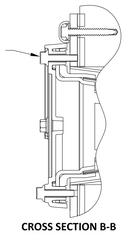
CONCRETE HEADWALL FOR SINGLE PIPE

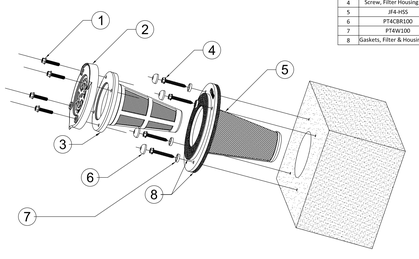
NOT TO SCALE

2 C9.5

#	ITEM	DESCRIPTION	QTY
1	Screw,Filter Cartridge Mtg.	¼-20x1" Screw, 304ss with 3/8" hex head	4
2	JF4-CV	Check Valve, Cartridge Cap	1
3	JF4-FC	Filter Cartridge Assembly	1
4	Screw, Filter Housing Mtg.	¼-20x1¼" Screw, 304ss with 5/16" hex head	4
5	JF4-HSS	Filter Housing Assembly	1
6	PT4CBR100	Plastic Protection Cap	4
7	PT4W100	Plastic Protection Washer	4
8	Gaskets, Filter & Housing Mtg.	Neoprene/Rubber/EPDM/Polymetric Blend	1 EA

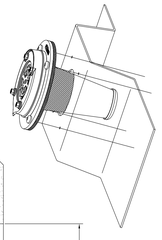
1 ¼-20 X 1" SCREW
Removable to maintain or replace the filter cartridge.

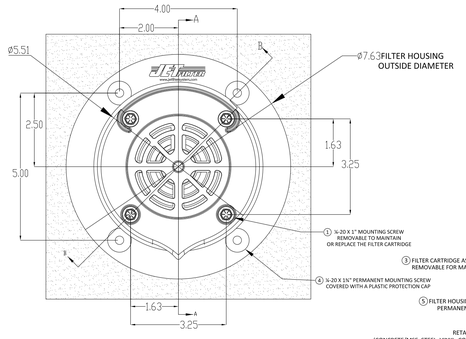


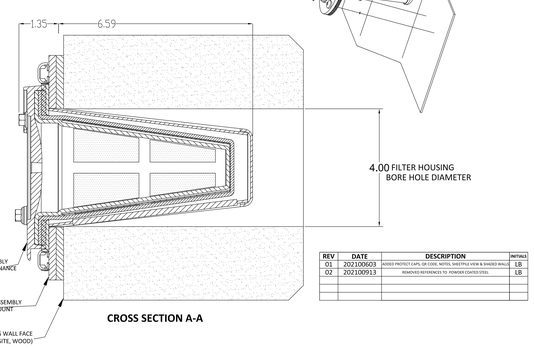


CROSS SECTION B-B
STANDARD HOUSING AND
FILTER ASSEMBLY MOUNTING

PILOT HOLE SIZES FOR MOUNTING:
CONCRETE/MSE: 3/16"
STEEL: 15/64" TO 7/32"
VINYL: PILOT HOLE NOT REQUIRED
COMPOSITE: PILOT HOLE NOT REQUIRED
WOOD: PILOT HOLE NOT REQUIRED







CROSS SECTION A-A

REV	DATE	DESCRIPTION	INITIALS
01	20210906S	ADDED PROTECT CAPS, OR CODE, NOTES, WEEPTEE VIEW & SHADED IMAGE	LB
02	20210091S	REMOVED REFERENCES TO POWDER COATED STEEL	LB

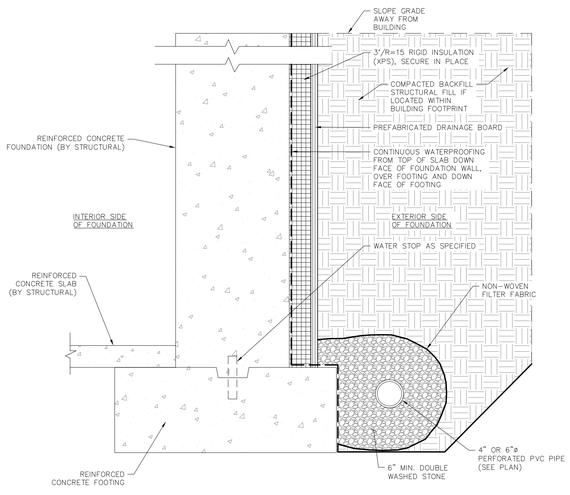
Filter Housing Faceplate: ¼-inch 316L Stainless Steel
Filter Housing Cone: 16 Gauge 316L Stainless Steel perforated with ¼-inch holes, staggered 5/16-inch
Filter Cartridge: UV Protected ABS
Filter Media: 26.03 square inches of flowable Tencate Mirafi Woven Fabrics FW300

NOTE: TO BE MANUFACTURED BY JET FILTER SYSTEM OR EQUAL. CORING THROUGH RETAINING WALL MATERIALS MAY BE REQUIRED.

RETAINING WALL DRAIN SYSTEM

NOT TO SCALE

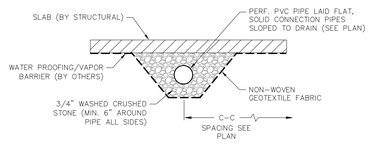
3 C9.5



FOUNDATION WALL SECTION

NOT TO SCALE

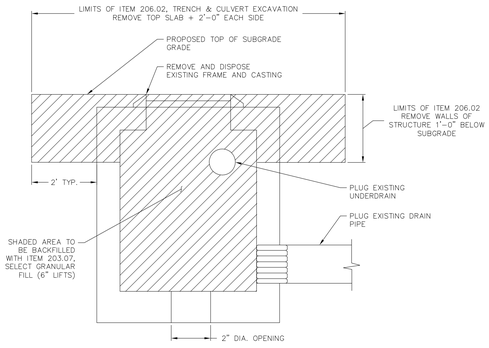
4 C9.5



SLAB UNDERDRAIN

NOT TO SCALE

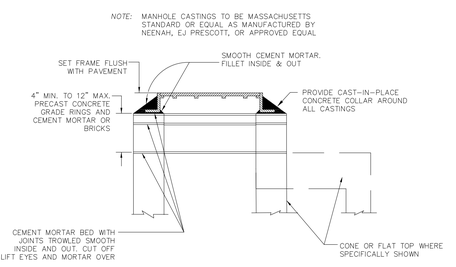
5 C9.5



ABANDON EXISTING DRAINAGE STRUCTURE IN PLACE-PLUG

SCALE: NONE

6 C9.5



STRUCTURE COVER/GRATE INSTALLATION DETAIL

SCALE: NONE

7 C9.5

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WINDIGO
ARCHITECTURE, LLC

914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921

WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC

4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035

Ph: (413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
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ISSUE

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	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47318
REGISTERED
PROFESSIONAL ENGINEER
3-8-2023

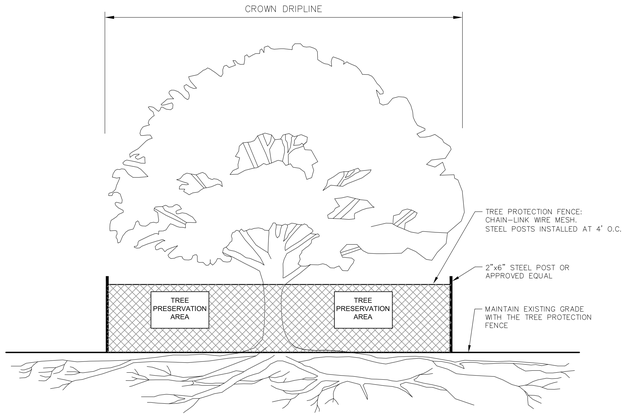
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.5

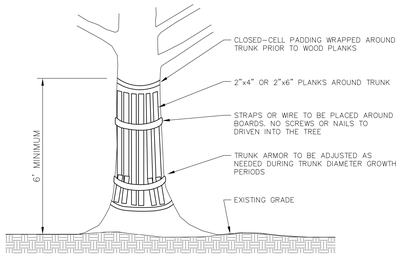
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TREE FENCING PROTECTION

NOT TO SCALE

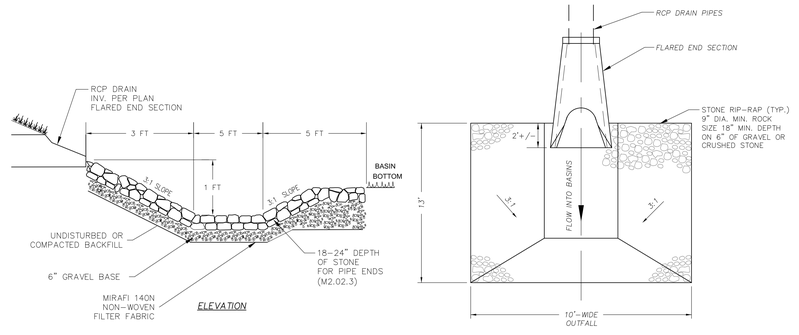
1 / C9.6



TREE ARMOR DETAIL

NOT TO SCALE

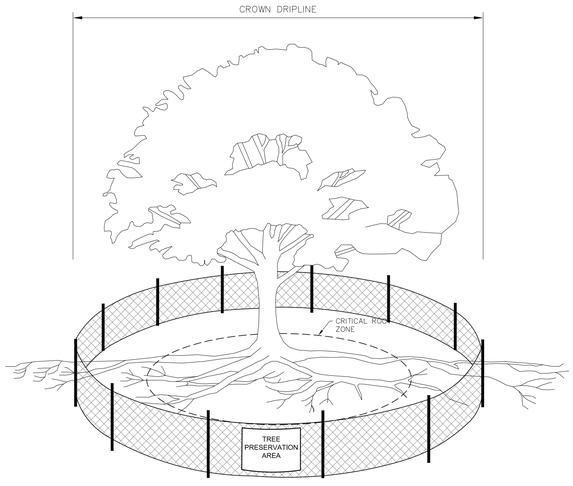
3 / C9.6



OUTLET EROSION PROTECTION DETAIL

SCALE: NONE

4 / C9.6



TREE PROTECTION ZONE

NOT TO SCALE

TREE PRESERVATION AREA

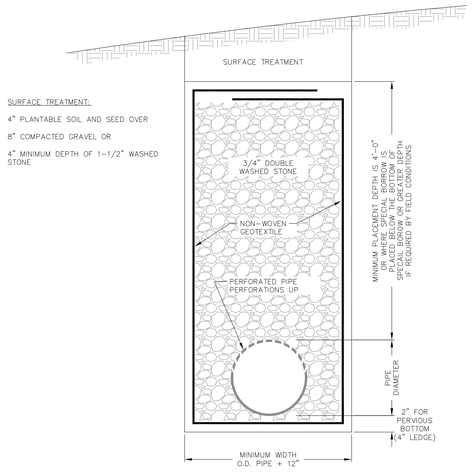
ENTRY PROHIBITED WITHOUT PRIOR AUTHORIZATION

Contact Deerfield Tree Warden Jason Miller with questions: 413-665-2036

TREE PRESERVATION AREA SIGN

NOT TO SCALE

2 / C9.6



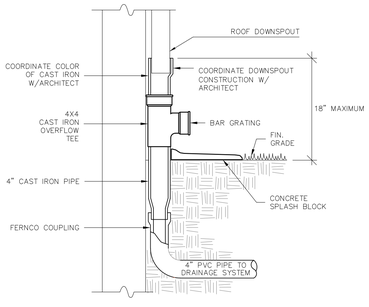
NOTES:

1. GEOTEXTILE FABRIC AS DESCRIBED IN SECTION M9.50
2. PIPE SHALL BE SET AT BOTTOM OF TRENCH FOR IMPERVIOUS BOTTOM.
3. SUBDRAIN LOCATED APPROXIMATELY AT INTERSECTION OF TANGENTS (SEE DWG. E102.1.0)
4. GRAVEL (AND SPECIAL BORROW WHERE REQUIRED) SHALL INTERSECT CRUSHED STONE FOR SUBDRAIN.

SUBDRAIN DETAIL

NOT TO SCALE

5 / C9.6



TYPICAL DOWNSPOUT CONNECTION

SCALE: NONE

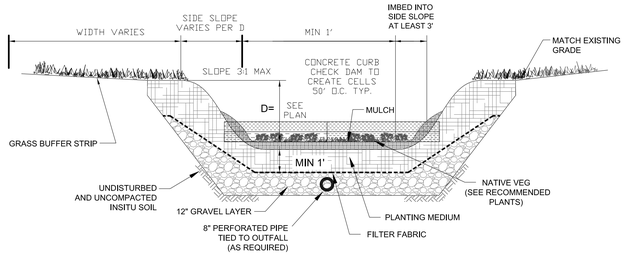
7 / C9.6

NOTE: CLAM DAM DETAIL SHALL BE USED BY CONTRACTOR IN AREAS OF HIGH GROUNDWATER OR AS DIRECTED BY THE ENGINEER TO PREVENT PROPAGATION ALONG EXCAVATED TRENCH.

CLAY DAM DETAIL

SCALE: NONE

6 / C9.6



TYPICAL DRY SWALE CROSS-SECTION (WITH CHECKDAM)

SCALE: NONE

8 / C9.6

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JESSE M. HOVENO
CIVIL
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3-8-2023

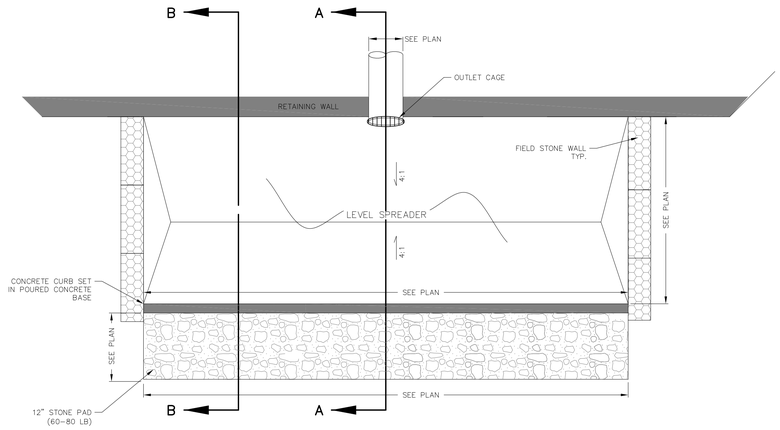
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.6

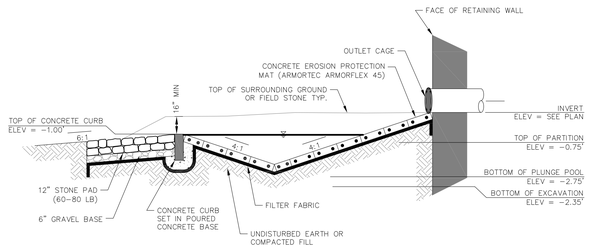
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LEVEL SPREADER & PLUNGE POOL OUTFALL DETAIL

NOT TO SCALE

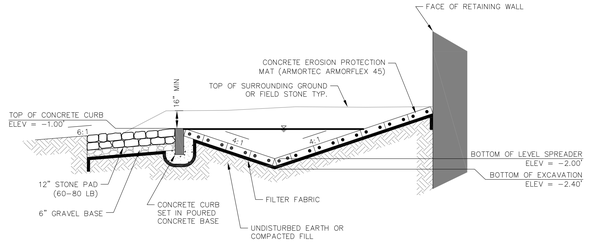
1 / C9.7



SECTION A-A (PLUNGE POOL)

NOT TO SCALE

NOTE: ALL ELEVATIONS RELATIVE TO DRAIN PIPE INVERT



SECTION B-B (LEVEL SPREADER)

NOT TO SCALE

NOTE: ALL ELEVATIONS RELATIVE TO DRAIN PIPE INVERT

STORMTRAP INSTALLATION SPECIFICATION

STORMTRAP MODULES SHALL BE MANUFACTURED ACCORDING TO SHOP DRAWINGS APPROVED BY THE INSTALLING CONTRACTOR AND ENGINEER. THE SHOP DRAWINGS SHALL INDICATE SIZE AND LOCATION OF ROOF OPENINGS AND INLET/OUTLET PIPE OPENINGS.

1. STORMTRAP SHALL BE INSTALLED IN ACCORDANCE WITH ASTM C891–90, STANDARD PRACTICE FOR INSTALLATION OF UNDERGROUND PRE–CAST CONCRETE UTILITY STRUCTURES. THE FOLLOWING ADDITIONS AND/OR EXCEPTIONS SHALL APPLY:

A. SPECIFICATIONS ON THE ENGINEER'S DRAWINGS SHALL TAKE PRECEDENT.

B. STORMTRAP MODULES SHALL BE PLACED ON LEVEL FOUNDATION (SEE DETAIL "A") WITH A 2'–0" OVERHANG ON ALL SIDES THAT SHALL BE INSTALLED BY CONTRACTOR.

C. THE STORMTRAP MODULES SHALL BE PLACED SUCH THAT THE MAXIMUM SPACE BETWEEN ADJACENT MODULES DOES NOT EXCEED 3/4". IF THE SPACE EXCEEDS 3/4", THE MODULES SHALL BE RESET WITH APPROPRIATE ADJUSTMENT MADE TO LINE AND GRADE TO BRING THE SPACE INTO SPECIFICATION.

D. THE PERIMETER HORIZONTAL JOINT OF THE STORMTRAP MODULES SHALL BE SEALED TO THE FOOTINGS WITH PREFORMED MASTIC JOINT SEALER ACCORDING TO ASTM C891–90, 8.8 AND 8.12.

E. ALL EXTERIOR JOINTS BETWEEN ADJACENT STORMTRAP MODULES SHALL BE SEALED WITH 1'–0" EXTERNAL SEALING BANDS CONFORMING TO ASTM C891–90 AND SHALL BE 1'–0" RUBBER BUTYL SEALANT AS APPROVED BY STORMTRAP. THE JOINT WRAP BUTYL EXTERIOR WRAP SHALL BE INSTALLED ACCORDING TO THE FOLLOWING INSTALLATION INSTRUCTIONS:

E. (CONTINUED)

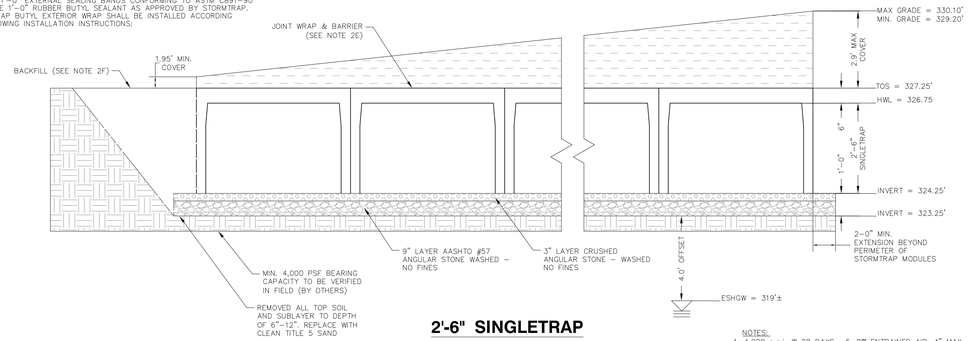
1. USE A BRUSH OR WET CLOTH TO THOROUGHLY CLEAN THE OUTSIDE SURFACE AT THE POINT WHERE THE JOINT WRAP IS TO BE APPLIED.

2. A RELEASE PAPER PROTECTS THE BUTYL SEALANT SIDE OF THE JOINT WRAP. WRAP THE BUTYL TAPE (BUTYL SIDE DOWN) AROUND THE STRUCTURE, REMOVING THE RELEASE PAPER AS YOU GO. PRESS THE JOINT WRAP FIRMLY AGAINST THE STORMTRAP MODULE SURFACE WHEN APPLYING.

F. THE FILL PLACED AROUND THE STORMTRAP UNITS MUST BE DEPOSITED ON BOTH SIDES AT THE SAME TIME AND TO APPROXIMATELY THE SAME ELEVATION. AT NO TIME SHALL THE FILL BEHIND ONE SIDE WALL BE MORE THAN 2'–0" HIGHER THAN THE FILL ON THE OPPOSITE SIDE. BACKFILL SHALL BE COMPACTED TO 95 % STANDARD PROTOR DENSITY OR OTHERWISE SPECIFIED BY ENGINEER. CARE SHALL BE TAKEN TO PREVENT ANY WEDGING ACTION AGAINIST THE STRUCTURE, AND ALL SLOPES BOUNDING OR WITHIN THE AREA TO BE BACKFILLED MUST BE STEPPED OR SERRATED TO PREVENT WEDGE ACTION. (REFERENCE ARTICLE 502.10 I.D.O.T. S.S.R.B.C.) CARE SHALL ALSO BE TAKEN AS NOT TO DISRUPT THE JOINT WRAP FROM THE JOINT DURING THE BACK FILL PROCESS. BACKFILL MATERIAL NOT TO EXCEED 120 PCF SOIL DENSITY OR 80 LBS PER FOOT OF LATERIAL SATURATED PRESSURE. RECOMMENDED BACKFILL TO CONSIST OF 3/4" AGGREGATE STONE (CA–7) OR APPROVED EQUAL AND SHALL CONFORM TO THE ABOVE DENSITY/LATERIAL SATURATED PRESSURE REQUIREMENTS.

STORMTRAP SPECIFICATION

1. TOTAL COVER: MIN. 0'–6" MAX. 3'–5" CONSULT STORMTRAP FOR ADDITIONAL COVER OPTIONS.
2. CONCRETE CHAMBER DESIGNED FOR AASHTO HS–20 WHEEL LOAD & APPLICABLE IMPACT.
3. FOR STRUCTURAL CALCULATIONS THE WATER TABLE IS ASSUMED TO BE 4'–0" BELOW GRADE. IF WATER TABLE IS LESS THAN 4'–0" BELOW GRADE, CONTACT STORMTRAP.
4. FOR STRUCTURAL CALCULATIONS THE VERTICAL AND LATERIAL SOIL PRESSURE IS ASSUMED TO BE 120 PCF.



2'-6" SINGLETRAP

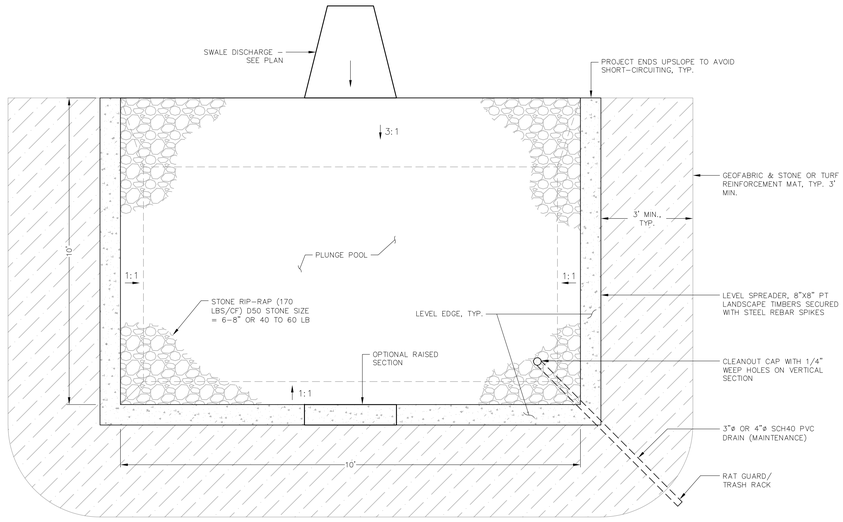
NOTE: UNDERGROUND VAULT SHALL BE STORMTRAP SINGLE TRAP OR APPROVED EQUAL.

NOTES:
1. 4,000 p.s.i. @ 28 DAYS, 5–8% ENTRAINED AIR, 4" MAX. SLUMP.
2. NET ALLOWABLE SOIL PRESSURE GREATER THAN OR EQUAL TO 4,000 PSF.
3. SOIL CONDITIONS TO BE VERIFIED ON SITE BY GEOTECH OF RECORD.
4. 4,000 p.s.i. MUST BE REACHED BEFORE STORMTRAP UNITS CAN BE INSTALLED.

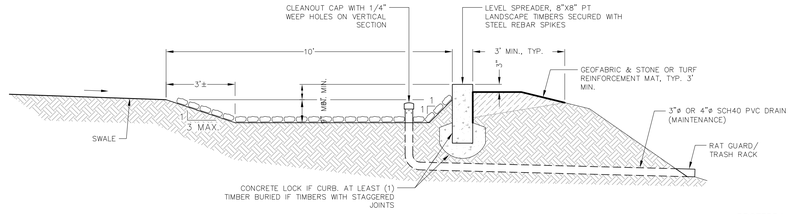
UNDERGROUND STORAGE VAULT WITH INFILTRATION

NOT TO SCALE

2 / C9.7



PLAN



ELEVATION

HAUL ROAD LEVEL SPREADER

SCALE: NONE

3 / C9.7

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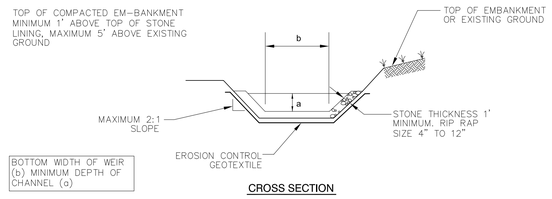
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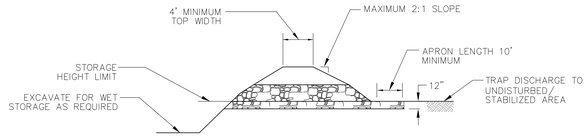
SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

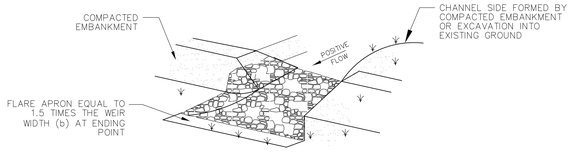
DRAWING NUMBER
C9.7



CROSS SECTION



PROFILE

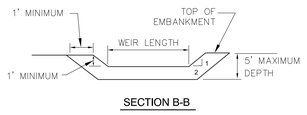


PERSPECTIVE

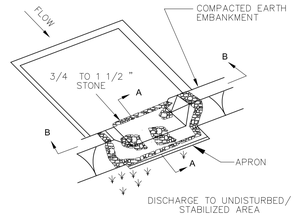
RIPRAP OUTLET SEDIMENT TRAP

SCALE: NONE

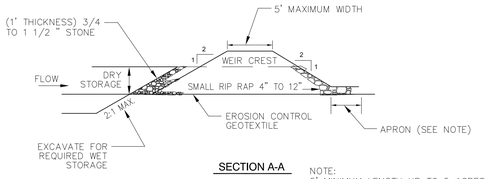
1 / C9.8



SECTION B-B



PERSPECTIVE VIEW



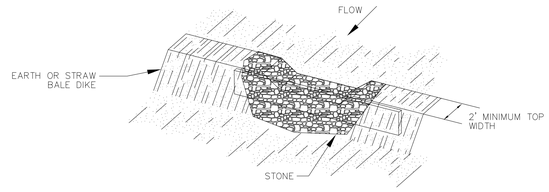
SECTION A-A

NOTE:
5' MINIMUM LENGTH UP TO 5 ACRES.
OVER 5 ACRES USE 10' MIN.

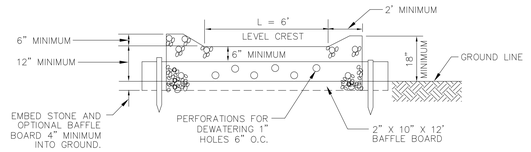
STONE OUTLET SEDIMENT TRAP

SCALE: NONE

2 / C9.8



PERSPECTIVE VIEW



CROSS SECTION

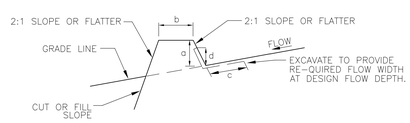
STONE OUTLET STRUCTURE

SCALE: NONE

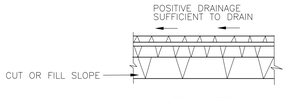
3 / C9.8

STABILIZATION AS REQUIRED.

	DIKE A (5AC OR LESS)	DIKE B (5-10AC)
a-DIKE HEIGHT	18"	30"
b-DIKE WIDTH	24"	36"
c-FLOW WIDTH	4'	6'
d-FLOW DEPTH	12"	24"



CROSS SECTION



PLAN VIEW

GRADE 0.5% MIN. 10% MAX.

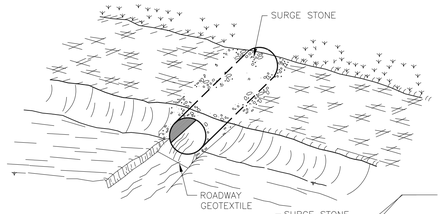
FLOW CHANNEL STABILIZATION

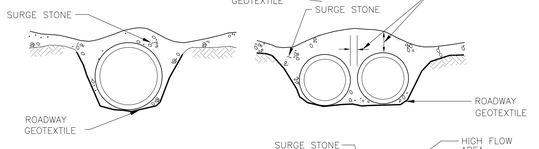
1. SEED AND COVER WITH STRAW MULCH.
2. SEED AND COVER WITH EROSION CONTROL MATTING OR LINE WITH SOD.
3. LINE WITH EROSION CONTROL GEOTEXTILE AND CLASS I RIP-RAP OR RECYCLED CONCRETE EQUIVALENT.
4. (TYPE B ONLY) LINE WITH EROSION CONTROL GEOTEXTILE AND CLASS II RIP-RAP.

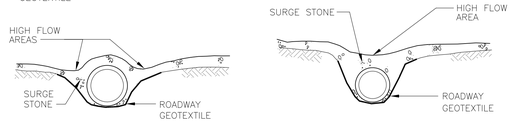
EARTH DIKE

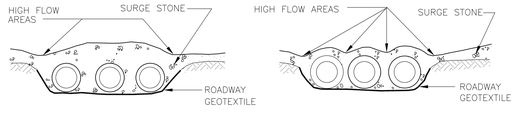
SCALE: NONE

4 / C9.8









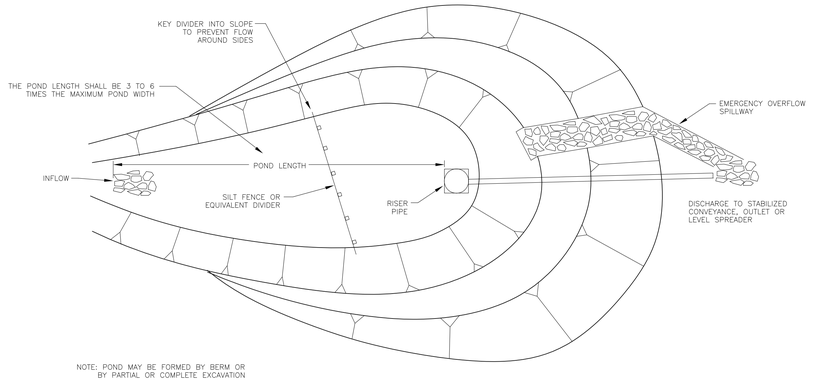
MULTIPLE PIPES

MULTIPLE PIPES

TEMPORARY ACCESS CULVERT

SCALE: NONE

5 / C9.8



NOTE: POND MAY BE FORMED BY BERM OR BY PARTIAL OR COMPLETE EXCAVATION

TEMPORARY SEDIMENT TRAP

SCALE: NONE

6 / C9.8

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WINDIGO
ARCHITECTURE, LLC
914 Mt. Kemble Avenue
Morristown, NJ 07960
p 973.425.7680
f 973.425.3921
WWW.WINDIGODESIGN.COM

SITE CONSULTANT

ProTerra
DESIGN GROUP, LLC
4 Bay Road, Bldg A
Suite 200
Hadley, MA 01035
Ph:(413)320-4918

Proposed Dining Hall
FOR
Eaglebrook School
LOCATED AT
271 Pine Nook Rd.
Deerfield, MA 01342

ISSUE

NO.	DATE	DESCRIPTION
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	08/04/22	DD PROGRESS SET
	08/08/22	DD PROGRESS SET
	09/09/22	DD PROGRESS SET
	10/28/22	DD PROGRESS SET
	02/28/23	DD PROGRESS SET
	03/08/23	NOI SUBMITTAL

PROGRESS SET
NOT FOR CONSTRUCTION

COMMONWEALTH OF MASSACHUSETTS
JESSE M. MORENO
CIVIL
No. 47816
REGISTERED PROFESSIONAL ENGINEER
3-8-2023

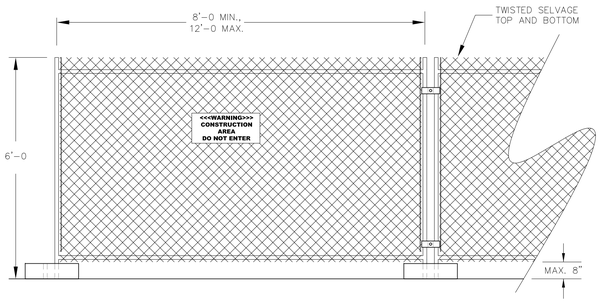
PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.8

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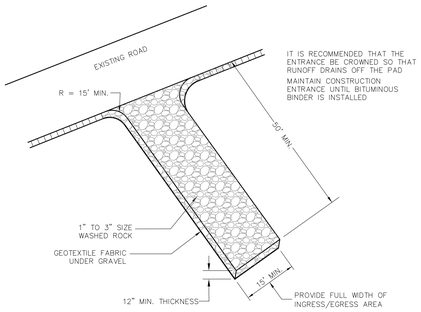


NOTES:
1. CHAIN LINK FABRIC TO BE MIN. 11 GAUGE, GALVANIZED. NO RUSTED OR EXCESSIVELY MALFORMED FABRIC.
2. FENCE BASES SHALL BE OF SUFFICIENT WEIGHT and/or SPREAD TO ADEQUATELY SUPPORT EACH PANEL.
3. PANEL-TO-PANEL CONNECTIONS SHALL BE MADE AT A MIN. TWO LOCATIONS PER CONNECTION UNLESS OTHERWISE APPROVED.
4. PROVIDE CONSTRUCTION WARNING SIGNAGE 50'O.C. ALONG FENCING INSTALLATION.

6' TEMPORARY CONSTRUCTION FENCE

SCALE: NONE

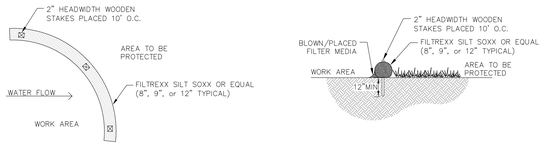
1 / C9.9



STABILIZED CONSTRUCTION ENTRANCE DETAIL

SCALE: NONE

2 / C9.9

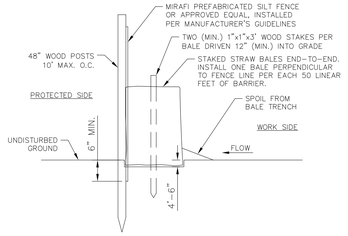


NOTES:
1. USE SILT SOXX OR APPROVED EQUAL WHERE CONDITIONS DO NOT ALLOW STAKES TO BE DRIVEN.
2. SILT SOXX FILL TO MEET FILTREXX SPECIFICATIONS AND APPLICATION REQUIREMENTS.
3. SILT SOXX COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER.

EROSION CONTROL BARRIER (MEDIA FILTER SOCK) DETAIL

SCALE: NONE

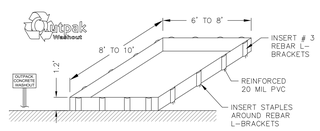
3 / C9.9



EROSION CONTROL BARRIER (SILT FENCE & HAYBALE) DETAIL

SCALE: NONE

4 / C9.9

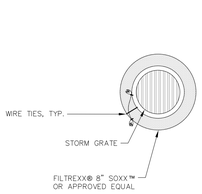


NOTES:
1. THE CONCRETE WASHOUT AREA SHALL BE INSTALLED PRIOR TO ANY CONCRETE PLACEMENT ON THIS PROJECT.
2. SIGNS SHALL BE PLACED AS NECESSARY TO CLEARLY INDICATE THE LOCATION OF THE CONCRETE WASHOUT.
3. THE CONCRETE WASHOUT AREA WILL BE REPLACED AS NECESSARY TO MAINTAIN CAPACITY FOR WASTE CONCRETE AND OTHER LIQUID WASTE.
4. WASHOUT RESIDUE SHALL BE REMOVED FROM THE SITE AND DISPOSED OF AT AN APPROVED WASTE SITE.
5. DO NOT MIX EXCESS AMOUNTS OF FRESH CONCRETE OR CEMENT ON-SITE.
6. DO NOT WASH OUT CONCRETE TRUCKS INTO STORM DRAINS, OPEN DITCHES, STREETS, OR STREAMS.
7. AVOID DUMPING EXCESS CONCRETE IN NON-DESIGNATED DUMPING AREAS.
8. LOCATE WASHOUT AREA AT LEAST 50' (15 METERS) FROM STORM DRAINS, OPEN DITCHES, OR WATERBODIES.
9. WASH OUT WASTES INTO THE OUTPACK WASHOUT AS SHOWN WHERE THE CONCRETE CAN SET, BE BROKEN UP, AND THEN DISPOSED OF PROPERLY.

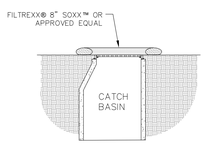
CONCRETE WASHOUT AREA DETAIL

SCALE: NONE

5 / C9.9

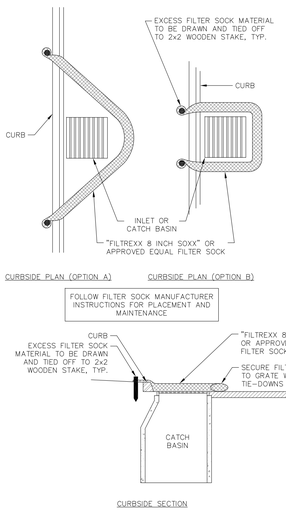


DRAIN INLET PLAN



DRAIN INLET SECTION

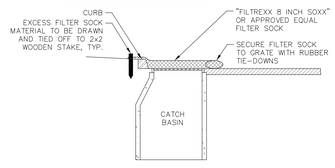
NOTES:
1. DETAILS SHOWN FROM FILTREXX SUSTAINABLE TECHNOLOGIES.
2. ALTERNATIVES ARE ACCEPTABLE WITH APPROVAL FROM ENGINEER OF RECORD AND CITY STAFF.
3. COMPOST MATERIAL TO BE DISPERSED ON SITE, AS DETERMINED BY ENGINEER.



CURBSIDE PLAN (OPTION A)

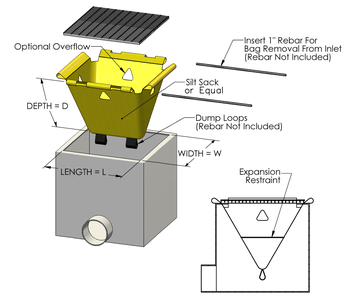
CURBSIDE PLAN (OPTION B)

FOLLOW FILTER SOCK MANUFACTURER INSTRUCTIONS FOR PLACEMENT AND MAINTENANCE



CURBSIDE SECTION

MEDIA FILTER SOCK INLET PROTECTION

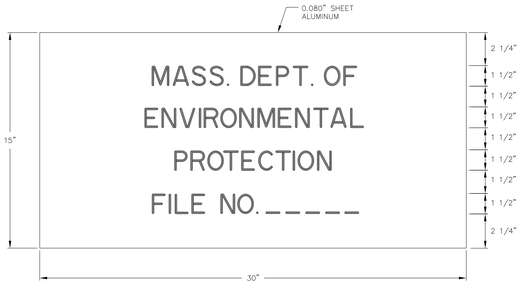


CATCH BASIN FILTER

INLET PROTECTION DETAILS

SCALE: NONE

6 / C9.9



COLORS — LEGEND – BLACK (NON-REFLECTORIZED); BACKGROUND – WHITE (REFLECTORIZED)

THE SIGN IS TO BE MOUNTED ON A MASSDOT HIGHWAY DIVISION STANDARD "P-5" POST.

NOTES:
1. THE SIGN IS PLACED ON ALL PROJECTS SUBJECT TO THE PROVISIONS OF THE MASSACHUSETTS WETLANDS PROTECTION ACT.
2. THE LOCATION OF THE SIGN IS TO BE DETERMINED BY THE ENGINEER.
3. SEE SPECIAL PROVISIONS FOR MANUFACTURE, MAINTENANCE, ERECTION AND REMOVAL RESPONSIBILITIES.
4. USE SERIES "D" FOR LETTERING.

WETLANDS PROTECTION ACT SIGN

NOT TO SCALE

2 / C9.9

EROSION CONTROL NOTES

1. OWNER/APPLICANT PROPOSES TO RENOVATE A VACANT SCHOOL BUILDING INTO RESIDENTIAL HOUSING WITH ASSOCIATED PARKING LOT AND WET/DRY UTILITY SERVICES.
2. TEMPORARY SILT FENCE EROSION CONTROL BARRIER SHALL BE MAINTAINED THROUGHOUT SITE CONSTRUCTION. STOCK PILE ON SITE 100 FT. OF SILT FENCE FOR EMERGENCY USE. TEMPORARY EROSION BARRIERS SHALL REMAIN IN PLACE UNTIL PERMANENT VEGETATIVE GROUND COVER IS ESTABLISHED.
3. THE CONTRACTOR SHALL PROVIDE 500± FT. OF TEMPORARY CHAIN LINK CONSTRUCTION FENCE TO BE PLACED AROUND ALL EQUIPMENT, WORK ZONES, AND MATERIAL LAYDOWN AREAS. PRICE OF FENCE TO BE CARRIED IN THE BID BY THE CONTRACTOR.
4. THE CONTRACTOR SHALL CHIP ALL BRUSH AND SLASH CUTTINGS ON SITE AND STOCKPILE THE CHIPS TO BE USED ON ALL UNSTABLE, DISTURBED AREAS DURING CONSTRUCTION AS TEMPORARY STABILIZATION MULCH. NO BURNING WILL BE ALLOWED ON SITE.
5. TEMPORARY STABILIZATION MUST BE PROVIDED TO ANY DISTURBED EARTH THAT IS OPENED UP IN ANY ONE LOCATION FOR MORE THAN 14 DAYS. CHIPS FROM LAND CLEARING, EROSION CONTROL BLANKETS, OR FAST GROWING RYE GRASSES MAY BE USED FOR TEMPORARY STABILIZATION AS REQUIRED.
6. STRIPPED TOPSOIL SHALL BE STOCKPILED AND PROTECTED WITH STRAW MULCH. ALL STOCKPILES SHALL HAVE AN APPROVED SILTATION BARRIER TOTALLY SURROUNDING THE PILE. THE PILE SHALL BE IN AN APPROVED UPLAND AREA A MINIMUM OF TWENTY-FIVE FEET FROM ALL RESOURCE AREAS.
7. THE PHASING AND SEQUENCING OF THE WORK FOR THE SITE PREPARATION FOR THE HOUSING CONVERSION CONSISTS OF INSTALLING TEMPORARY CONSTRUCTION EXIT POINTS; INSTALLING TEMPORARY EROSION AND SEDIMENTATION CONTROL BARRIERS; CLEARING AND ROUGH GRADING AS NEEDED; EXCAVATION/FILL FOR BUILDING FOUNDATION WORK AT ELEVATOR ENTRANCE; EXCAVATION FOR UTILITIES; BACK FILL FOUNDATIONS AND UTILITY TRENCHES; VERTICAL BUILDING INSTALLATION AT ELEVATOR ENTRANCE; FINAL GRADING AND STABILIZATION OF DISTURBED AREAS; CONSTRUCTION OF CURB, DRIVES, AND PARKING AREAS; LOAM AND SEED DISTURBED AREAS OUTSIDE PAVEMENT; INSTALL FINAL LANDSCAPING AND CLEANUP. THE ESTIMATED TIME FOR COMPLETION OF THE WORK IS APPROXIMATELY **SIX (6) TO TWELVE (12) MONTHS.**
8. ALL DISTURBED AREAS SHALL BE PERMANENTLY ESTABLISHED PER THE LANDSCAPING PLAN OR WITH A NATIVE VEGETATIVE GROUND COVER AT THE CONCLUSION OF CONSTRUCTION. GRADED AREAS SHALL BE PROTECTED WITH STRAW MULCH UNTIL A GOOD VEGETATIVE COVER IS ESTABLISHED.
9. THE TOTAL IMPACT AREA OF THE DISTURBED CONSTRUCTION SITE IS BOUNDED BY THE "LIMIT OF WORK" AS SHOWN HEREON. THE MAXIMUM AREA OF DISTURBANCE WITHIN THE LIMIT OF WORK IS APPROXIMATELY **9.4± ACRES**. THE PROJECT IMPACT AREA IS ABOVE THE EXEMPTION THRESHOLD OF 43,560 SQUARE FEET IN 40 CFR PARTS 9, 122-124 AND THEREFORE IS SUBJECT TO REGULATION UNDER THE EPA NPDES GENERAL CONSTRUCTION PERMIT PROGRAM.
10. CONTRACTOR SHALL PREPARE A STORMWATER POLLUTION PREVENTION PLAN (SWPPP) AND FILE FOR COVERAGE UNDER THE NATIONAL POLLUTANT DISCHARGE ELIMINATION SYSTEM (NPDES) GENERAL PERMIT FOR DISCHARGES FROM CONSTRUCTION ACTIVITIES.
11. THE PROJECT OWNER'S GENERAL CONTRACTOR SHALL CONDUCT ALL SITE DEVELOPMENT WORK IN A MANNER THAT DOES NOT EXCEED THE LIMITS OF WORK SHOWN ON THE PLANS. ADDITIONALLY, THE PROJECT OWNER'S GENERAL CONTRACTOR SHALL CONDUCT ALL CONSTRUCTION ACTIVITIES IN A MANNER THAT DOES NOT RESULT IN STORM WATER DISCHARGES WITH AN ADVERSE IMPACT ON ANY RESOURCE AREAS OR DOWNSTREAM PROPERTIES.
12. UPON COMPLETION OF THE PROJECT, THE CONTRACTOR SHALL REMOVE ALL ACCUMULATED SILT FROM BEHIND SILTATION BARRIERS AND DISPOSE OF SILT EVENLY IN UPLAND AREAS. REMOVE ALL EROSION CONTROL DEVICES WHEN A GOOD VEGETATIVE COVER IS ESTABLISHED.

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ARCHITECTURE, LLC
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PROGRESS SET
NOT FOR CONSTRUCTION

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PROJECT NUMBER
22008

SCALE
SEE PLAN

DRAWING NAME
CIVIL SITE DETAILS

DRAWING NUMBER
C9.9

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